

*A Note on the Application
of Large Mass and Stiffness Techniques
for Multi-support Excitations*



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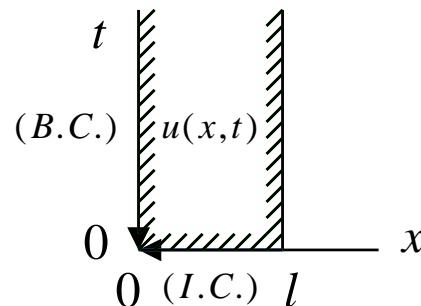
Why the Results Diverge by Using MSC/NASTRAN ?

- **Rayleigh Damping Proportional to Mass Only**

$$[C] = \alpha[M] + \beta[K], \quad \beta = 0$$

- **Large Stiffness Method Considering the Former Modes**
- **Discontinuity Between Boundary (B.C.) and Initial Conditions(I.C.)**

$$\begin{array}{ll}
 t \rightarrow 0 & x \rightarrow 0 \\
 u(0, t) \neq u(x, 0) & \\
 (B.C.) & (I.C.)
 \end{array}$$



Sources of Divergence in Modal Dynamics by Crandall

- **S. H. Crandall and A. Yildiz , Transactions of the ASME, 1962**

Beam modal (Euler-Bernoulli, Rayleigh, Timoshenko and shear beam)

Damping mechanism (Rayleigh, viscoelastic, internal, external damping)

Random response (primary and secondary fields)

Transient response is not available

?

Loading type (boundary force and support excitations are not available)

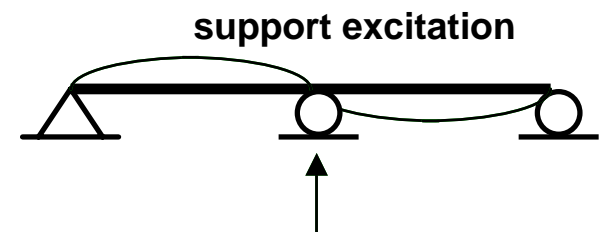
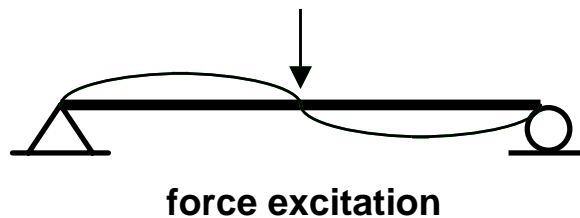
Sources of Divergence in This Paper

- **Rayleigh damping proportional to mass only for support motion**
- **Series solution without considering the boundary effect for support motion**
- **Large stiffness technique without considering the contributions of high frequency modes**
- **Discontinuity between boundary and initial conditions**

New Concept of Modal Participation Factor(MPF)

- **Modal displacement governs the MPF for body force excitations**
- **Modal displacement governs the MPF for boundary force excitations**
- **Modal reaction governs the MPF for boundary support excitations**

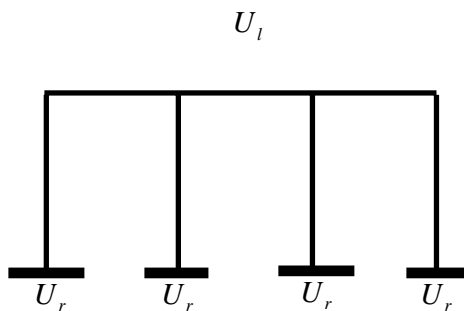
- $$\Gamma_{ij} = \frac{R_{ij}}{(-\omega_i^2)}$$



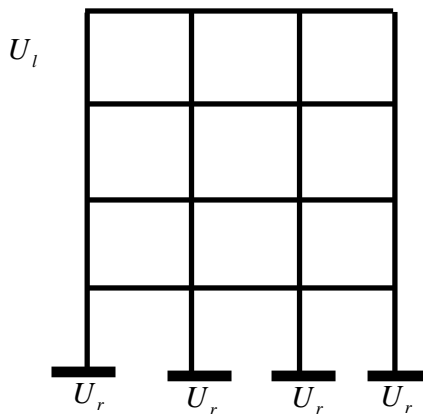
Discrete MDOF System

Governing Equation :

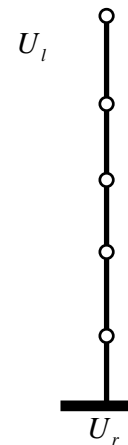
$$\begin{bmatrix} M_{ll} & M_{lr} \\ M_{rl} & M_{rr} \end{bmatrix} \begin{Bmatrix} \ddot{U}_l \\ \ddot{U}_r \end{Bmatrix} + \begin{bmatrix} C_{ll} & C_{lr} \\ C_{rl} & C_{rr} \end{bmatrix} \begin{Bmatrix} \dot{U}_l \\ \dot{U}_r \end{Bmatrix} + \begin{bmatrix} K_{ll} & K_{lr} \\ K_{rl} & K_{rr} \end{bmatrix} \begin{Bmatrix} U_l \\ U_r \end{Bmatrix} = \begin{Bmatrix} P_l \\ P_r \end{Bmatrix}$$



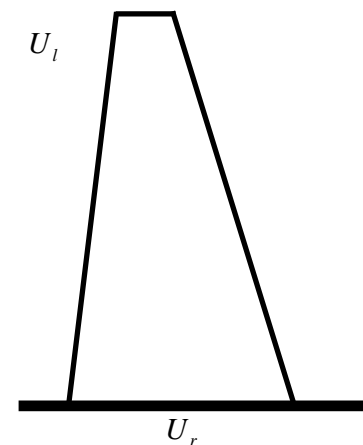
Bridge



Building



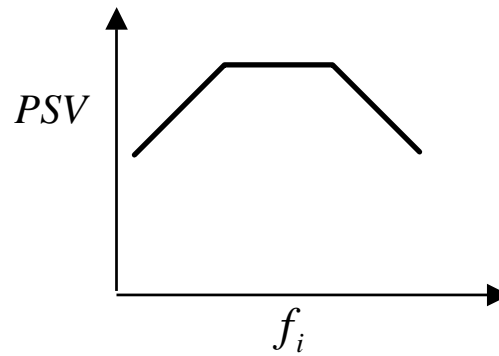
Model



Dam

Support Motion Problems

- Response ← Superposition of Modal Response (ABS, CQC, SRSS)
← **Modal Participation Factor Γ_i**
+ Response Spectrum



Formulations

- Governing Equation:**

$$\begin{bmatrix} M_{ll} & M_{lr} \\ M_{rl} & M_{rr} \end{bmatrix} \begin{Bmatrix} \ddot{U}_l \\ \ddot{U}_r \end{Bmatrix} + \begin{bmatrix} C_{ll} & C_{lr} \\ C_{rl} & C_{rr} \end{bmatrix} \begin{Bmatrix} \dot{U}_l \\ \dot{U}_r \end{Bmatrix} + \begin{bmatrix} K_{ll} & K_{lr} \\ K_{rl} & K_{rr} \end{bmatrix} \begin{Bmatrix} U_l \\ U_r \end{Bmatrix} = \begin{Bmatrix} P_l \\ P_r \end{Bmatrix}$$

- Decomposition:**

$$\begin{Bmatrix} U_l \\ U_r \end{Bmatrix} = \begin{Bmatrix} U^s \\ U^d \end{Bmatrix}$$

- Quasi-static Part:**

$$\{U^s\} = \begin{bmatrix} K_{ll}^{-1} K_{lr} \\ I \end{bmatrix} \begin{Bmatrix} U_r \end{Bmatrix} = \begin{bmatrix} G_1 & \dots & G_j & \dots & G_{N_r} \end{bmatrix}^T \begin{Bmatrix} U_{r1}(t) \\ M \\ U_{rj}(t) \\ M \\ U_{rN_r}(t) \end{Bmatrix}$$

Modal Formulations

- **Mode superposition** $\mathbf{u}^d = [\Phi_{NL}] \mathbf{q}^d$
- **Modal Equation** $\ddot{q}_i + 2\xi_i \omega_i \dot{q}_i + \omega_i^2 q_i = -\Gamma_{ij} \ddot{u}_{rj}(t)$
- **Modal Participation Factor (Conventional Method)** $\Gamma_{ij} = \frac{\mathbf{R}^T \phi_i}{\phi_i^T \mathbf{M} \phi_i} \mathbf{U}_{rj}$
- **Modal Participation Factor (Modal Reaction Method)** $\Gamma_{ij} = \frac{R_{ij}}{(-\omega_i^2)}$

Methods of Solution for Multi-Support Motions

- **Large Mass Technique (1983) —————> Multi-support Motions(1992)**
- **Large Stiffness Technique (1983) —————> Multi-support Motions(1992)**
- **Cesaro Sum (1890) —————> Base Shear Force (1992)**
- **Quasi-static Decomposition (1950) —————> Discrete System (1975)**
- **Stokes' Transformation (1880) —————> Base Shear Force (1993)**

Methods of Solution for Multi-Support Motions

- **Mode Superposition (Series Solution)**
- **Large Mass Technique (Including Rigid Body Modes)**
- **Large Stiffness Technique (Including High Frequency Modes)**
- **Cesaro sum**
- **Quasi-static Decomposition (Mindlin)**
- **Stokes' Transformation**

Capabilities of Multi-support Excitation in FEM Packages

- **MSC/NASTRAN**
 - Large mass technique
 - Large stiffness technique
- **ABAQUS**
 - Single base excitation(* BASE MOTION), modal reaction available
- **SAP90, ETABS**
 - ?
- **MLTDYN(NTUCE)**
 - Modal reaction method

Large Stiffness Method and Large Mass Method

Large mass

Large stiffness

Large mass

Large stiffness

Large Stiffness Technique

- **Large Stiffness Technique (Without Including High Frequency Modes)**
Divergence (slope, moment and shear force)
- **Large Stiffness Technique (Including High Frequency Modes)**
Boundary effect (moment and shear force)
- **Large Stiffness Ratio 10^6**
- **The Additional High Frequency Modes Display the Boundary Terms**

Large Mass Technique

- **Large Mass Technique (Without Including Rigid Body Modes)**
Divergence (slope, moment and shear force)
- **Large Mass Technique (Including Rigid Body Modes)**
Boundary effect (moment and shear force)
- **Large Mass Ratio 10^6**
- **The Rigid Body Modes Display the Boundary Terms**

*Results of Large Stiffness Method
Without Considering High Frequency Modes*

Displacement

Moment

Slope

Shear force

*Results of Large Stiffness Method
Including High Frequency Modes*

Displacement

Moment

Slope

Shear force

Rayleigh Damping Model

- Rayleigh Damping Proportional to Mass Only**
 $[C] = \alpha[M] + \beta[K], \beta = 0$
 α, β fixed
 $\xi_1 = 0.05$ fixed

Divergence Due to Mass Proportional Damping

Displacement

Moment

Slope

Shear force

*Divergence of Acceleration Profile Due to Discontinuity Between
Boundary and Initial Conditions at $t=0$ second*

**Displacement
(Initial condition)**

**Velocity
(Initial condition)**

**Acceleration
(Divergence)**

*Displacement and Acceleration Histories at the Middle Point $x/l=0.5$
Due to Discontinuity Between Boundary and Initial Conditions*

**Analytical
results**

Displacement

**Large stiffness method
(with high freq. modes)**

Displacement

**Large stiffness method
(no high freq. modes)**

Displacement

Acceleration

Acceleration

Acceleration

Relations of Series Representation, Large Stiffness Technique, Cesaro Sum, Quasi-static Decomposition and Stokes' Transformation

Motivations of Quasi-static Decomposition and Stokes' Transformation

Quasi-static decomposition

$$u(x,t) = \underline{U(x,t)} + \sum_{n=0}^N q_n(t) u_n(x)$$

(Physical meaning)

Differentiation

$$u'(x,t) = U'(x,t) + \sum_{n=0}^N q_n(t) u'_n(x)$$

$$q_n(t) = c_n(t) + \bar{q}_n(t)$$

$$U(x,t) \cong - \sum_{n=0}^N c_n(t) u_n(x)$$

Stokes' transformation

$$u(x,t) \cong \sum_{n=0}^N \bar{q}_n(t) u_n(x) \xrightarrow{\text{Asymptotic analysis}}$$

$$= U(x,t) + \sum_{n=0}^N c_n(t) u_n(x) + \sum_{n=0}^N \bar{q}_n(t) u_n(x)$$

Integration

$$u'(x,t) = \sum_{n=0}^N \overset{\text{ } \nearrow -\infty}{\underline{b_n(t) u'_n(x)}} + \sum_{n=0}^N \overset{\text{ } \nearrow \infty + F.P.}{\bar{q}_n(t) u'_n(x)}$$

(Mathematical way)

$$= U'(x,t) + \sum_{n=0}^N \underset{\text{ } \searrow -\infty}{c_n(t) u'_n(x)} + \sum_{n=0}^N \underset{\text{ } \searrow \infty + F.P.}{\bar{q}_n(t) u'_n(x)}$$

$$\sum_{n=0}^N$$

$$b_n(t) u'_n(x) =$$

**Series representation
for distribution on boundary**

*Three Analytical Ways and Two Simulation Techniques
to Introduce the Quasi-static Part*

- **By Solving Boundary Value Problem Directly**
Quasi-static decomposition method (Mindlin and Goodman)
- **By Integrating the Secondary Field Derived from Stokes' Transformation**
Boundary terms are available
- **By Adding and Subtracting Technique Using Asymptotic Analysis**
Series representation (Eringen and Suhubi, Yeh and Liaw)
- **Large Mass Technique (MSC/NASTRAN) --- Rigid Body Modes**
- **Large Stiffness Technique (MSC/NASTRAN) --- High Frequency Modes**

Cesaro Regularization Technique

- Series Solution(Partial Sum)**

$$s_0 = a_0$$

$$s_1 = a_0 + a_1$$

$$s_2 = a_0 + a_1 + a_2$$

$$\vdots$$

$$s_{n-1} = a_0 + a_1 + a_2 + \dots + a_{n-1}$$

$$(partial \ sum) \ s_n = a_0 + a_1 + a_2 + \dots + a_{n-1} + a_n \ (divergent, \ n \rightarrow \infty)$$

$$\frac{s_0 + s_1 + \dots + s_{n-1} + s_n}{n+1} = a_0 + \frac{n}{n+1}a_1 + \frac{n-1}{n+1}a_2 + \dots + \frac{2}{n+1}a_{n-1} + \frac{1}{n+1}a_n \ (convergent, \ n \rightarrow \infty)$$

$$(Cesaro \ sum) \ S_n = \frac{1}{n+1} \sum_{k=0}^n (n-k+1) a_k \ (moving \ average)$$

Stokes' Transformation

- **Term by Term Differentiation Is Not Always Legal**
- **Boundary Term Is Present for Some Cases**

$$f'(x) = \frac{d}{dx} \left[f(x) \right] = \frac{d}{dx} \left[\sum_{k=0}^n c_k u_k(x) \right] = \sum_{k=0}^n c_k u'_k(x) + \underbrace{\sum_{k=0}^n b_k u'_k(x)}_{\text{Boundary term}}$$

if $\sum_{k=0}^n b_k u'_k(x) \neq 0$

Boundary term

- **Term by Term Differentiation Is Legal**

if $\sum_{k=0}^n b_k u'_k(x) = 0$

*The Series Representation Solutions of the Three Analytical
Formulations*

The Proper Use of Each Method for Different Structures

Conclusions

- **Sources of Divergence Have Been Identified**
- **New Point of View for Modal Participation Factor has been developed**
Physical meaning
To save CPU time (100 : 1)
- **New Method for Multi-support Motion --- Stokes' Transformation**
Free from calculating quasi-static solution
Accelerate convergence rate
- **Large Stiffness and Large Mass Technique Have Been Tested**
Boundary effect of moment and shear diagrams should be noted