



ELASTOPLASTIC COMPUTING SAINT-VENANT FLEXURE-TORSION AND WARPING TORSION IN THREE DIMENSIONS

Hong-Ki Hong and Hsien-He Huang*

National Taiwan University

1. Motivation: computing by two cycles & Muto Kiyoshi
2. Objective: Saint-Venant's flexure-torsion & warping torsion
3. Various prismatic cross sections (e.g., ellipse, rectangle, thin-walled, singly-connected, multi-connected)
4. Different loading types
5. CM material & structural models of flow elastoplasticity
6. Conclusions

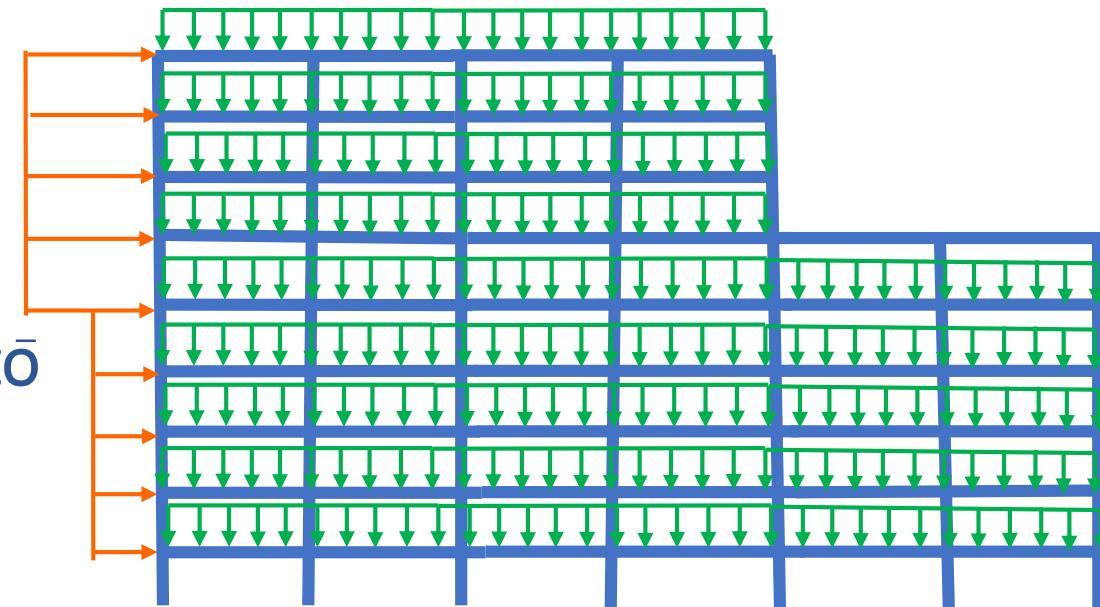
Motivation: Computing in Building Frame Design

Hardy Cross (1929) Statically indeterminate beam and frame

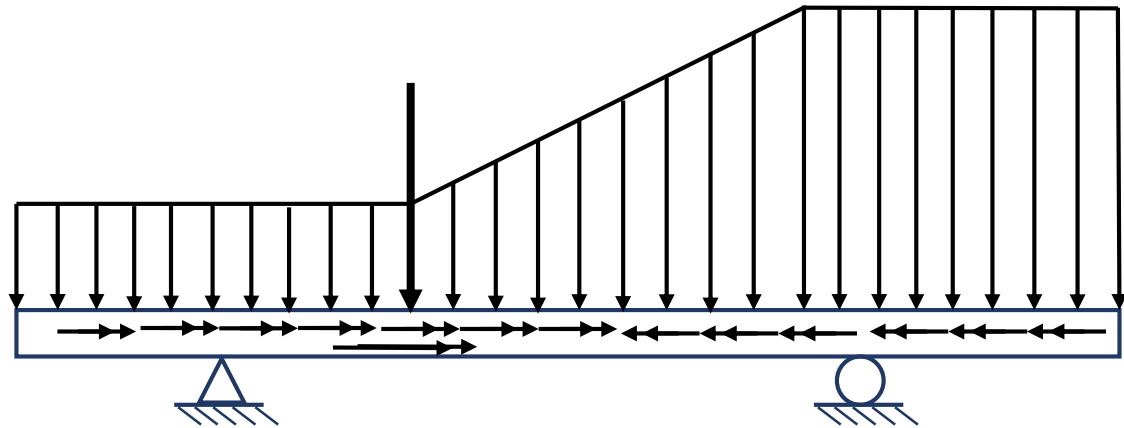
Vertical load (gravity): Two-Cycle Moment Distribution method

Horizontal load (earthquake, wind): 武藤清 Muto Kiyoshi method

only **flexure** is considered



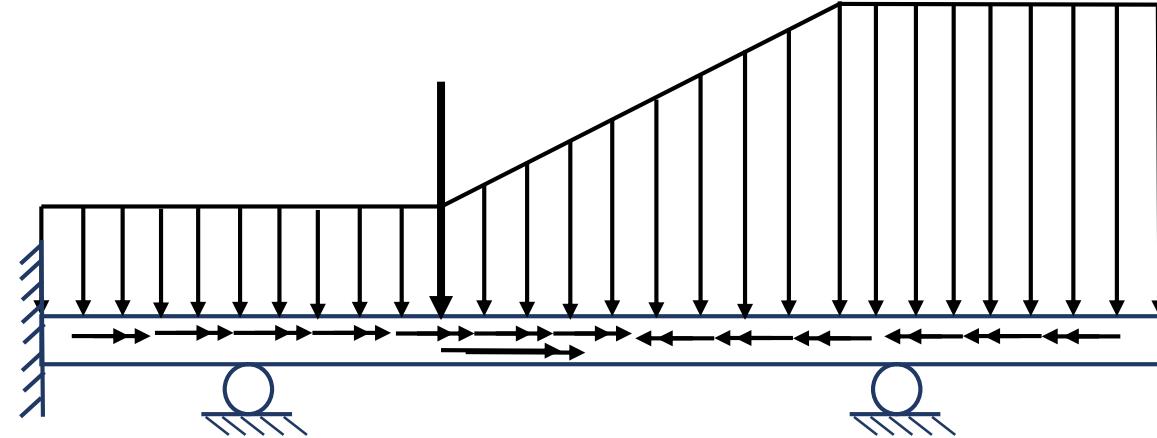
✓ Statically determinate



↓↓↓ Distributed loads

↓ Concentrated loads

✗ Statically indeterminate



→→→→ Distributed torsional loads

→ Concentrated torsional loads

Flexure



Saint-Venant's flexure-torsion theory



Holomorphic functions

Complex-valued harmonic functions

Holomorphic theory

→ Elegant but twisting rate ϕ' restrained to be constant
Statically determinate



Warping torsion theory

Warping torsion theory

→ Twisting angle ϕ allowed to vary
Statically indeterminate



國立臺灣大學
National Taiwan University

HSV

ACMT

WCCM-PANACM
VANCOUVER 2024

Objective

The Saint-Venant's flexure-torsion theory can display stresses and warping displacements on prismatic cross sections.

But it cannot demonstrate variations along the axial direction.

The warping torsion theory can display variations along the axial direction.

But it cannot display stresses and warping displacements on sections.

Now we seek a approach which can display stresses and warping displacements on sections **and** display variations along the axial direction **simultaneously**.

Saint-Venant's semi-inverse method

Saint-Venant's flexure-torsion theory

$$\text{GE} \quad -\nabla^2 S = \left[\frac{\nu E}{1+\nu} \kappa'_1 x_1 + \frac{df_2}{dx_1} \right] + \left[\frac{\nu E}{1+\nu} \kappa'_2 x_2 + \frac{df_1}{dx_2} \right] + 2G\phi'_3 \quad \text{for } R \subset \mathbb{R}^2$$

$$\text{BC} \quad \frac{ds}{ds} = \frac{dx_1}{ds} \left[-E\kappa'_1 \frac{x_2^2}{2} + f_2(x_1) \right] - \frac{dx_2}{ds} \left[E\kappa'_2 \frac{x_1^2}{2} + f_1(x_2) \right] \quad \text{for } \partial R$$

$$\text{GE} \quad \nabla^2 W = 0 \quad \text{for } R \subset \mathbb{R}^2$$

$$\text{BC} \quad \frac{dW}{ds} = \kappa'_1 [(1+\nu)x_2^2 - \nu x_1^2] n_2 + \kappa'_2 [\nu x_2^2 - (1+\nu)x_1^2] n_1 + \phi'_3 (x_2 n_1 - x_1 n_2) \quad \text{for } \partial R$$

E : Young's modulus

G : Shear modulus

ν : Possion's ratio

S : stress function

W : warping function

F_1 : concentrated transverse force in x_1 direction

F_2 : concentrated transverse force in x_2 direction

T : concentrated torque

s : arc length coordinate

n : normal coordinate

κ_1 : curvature of x_1 direction ; $\kappa'_1 = \frac{F_2}{EI_{11}}$

κ_2 : curvature of x_2 direction ; $\kappa'_2 = \frac{F_1}{EI_{22}}$

f_1 & f_2 : arbitrary function

ϕ_3 : twist angle of x_3 direction

ϕ'_3 : rate of twist per unit length



Notation

x_1, x_2, x_3 : location

w : displacement in z-direction

F_1, F_2, T_D : Free end concentrated load
& torsion

ϕ : angle of twist

I : second axial moment of area

T_s : Saint-Venant torsion

$I_{ww} = \int w^2 dF$: sectorial moment of inertia

T_w : warping torsion

f : warping factor

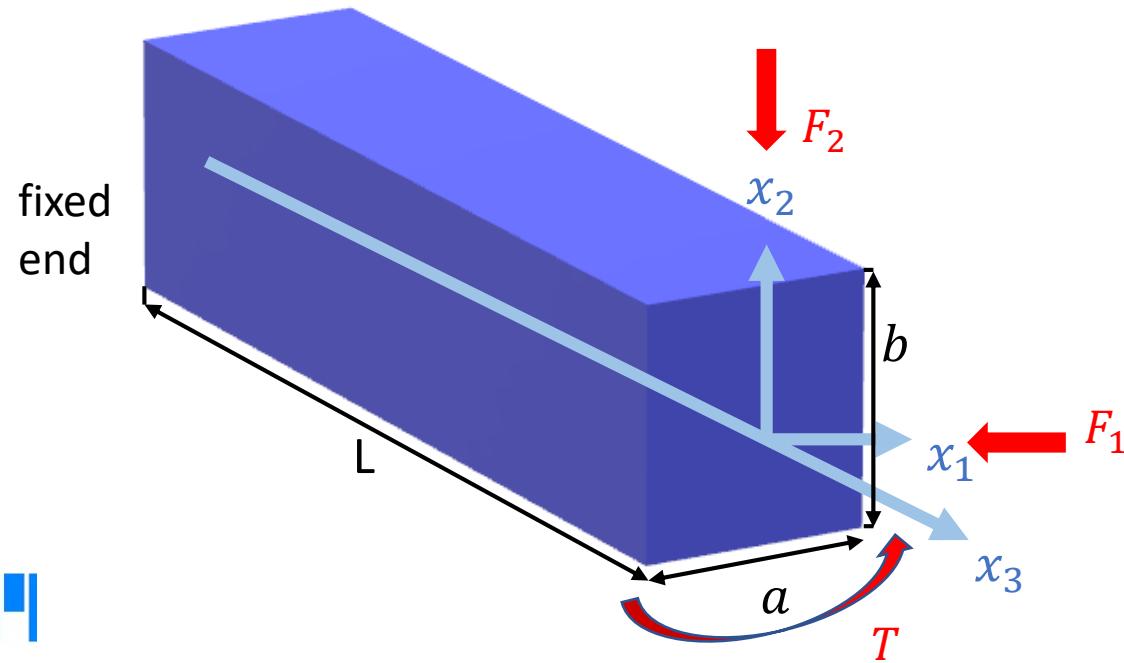
K : section factor

E : Young's modulus

G : Shear modulus of elasticity

Saint-Venant's flexure-torsion theory

GE	$-\nabla^2 S = \left[\frac{vE}{1+v} \kappa'_1 x_1 + \frac{df_2}{dx_1} \right] + \left[\frac{vE}{1+v} \kappa'_2 x_2 + \frac{df_1}{dx_2} \right] + 2G\phi'_3$	for $R \subset \mathbb{R}^2$
BC	$\frac{ds}{ds} = \frac{dx_1}{ds} \left[-E\kappa'_1 \frac{x_2^2}{2} + f_2(x_1) \right] - \frac{dx_2}{ds} \left[E\kappa'_2 \frac{x_1^2}{2} + f_1(x_2) \right]$	for ∂R
GE	$\nabla^2 W = 0$	for $R \subset \mathbb{R}^2$
BC	$\frac{dW}{ds} = \kappa'_1 [(1+v)x_2^2 - vx_1^2] n_2 + \kappa'_2 [vx_2^2 - (1+v)x_1^2] n_1 + \phi'_3 (x_2 n_1 - x_1 n_2)$	for ∂R



Warping torsion theory¹

$$T = T_S + T_w$$

$$T_S = GK\phi_3' \quad T_w = -(EI_{ww} \phi_3'')'$$

$$EI_{ww} \phi_3'''' - GK\phi_3'' = t_D$$

$$\phi_3 = C_1 + C_2 x_3 f + C_3 e^{-x_3 f} + C_4 e^{+x_3 f}$$

$$warping factor f = \sqrt{\frac{Gk}{EI_{ww}}}$$

BC $\phi_3(x_3 f = 0) = 0$
 $\phi_3'(x_3 f = 0) = 0$
 $\phi_3'(x_3 f = \infty) = 0$
 $T(x_3 f = \infty) = T_D$

$$\phi_3 = \frac{T_D}{GKf} (-1 + x_3 f + e^{-x_3 f})$$



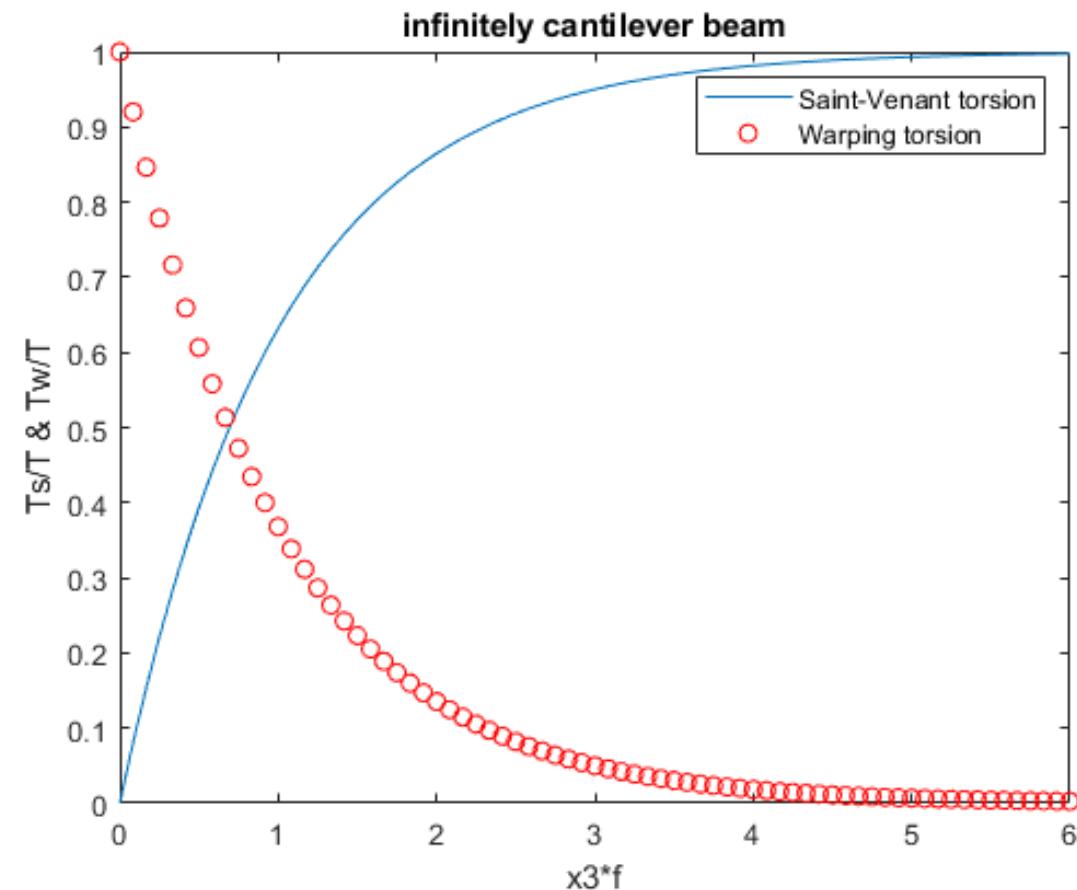
Warping torsion theory

Saint-Venant torsion vs. warping torsion

$$\phi_3 = \frac{T_D}{GKf} (-1 + x_3 f + e^{-x_3 f})$$

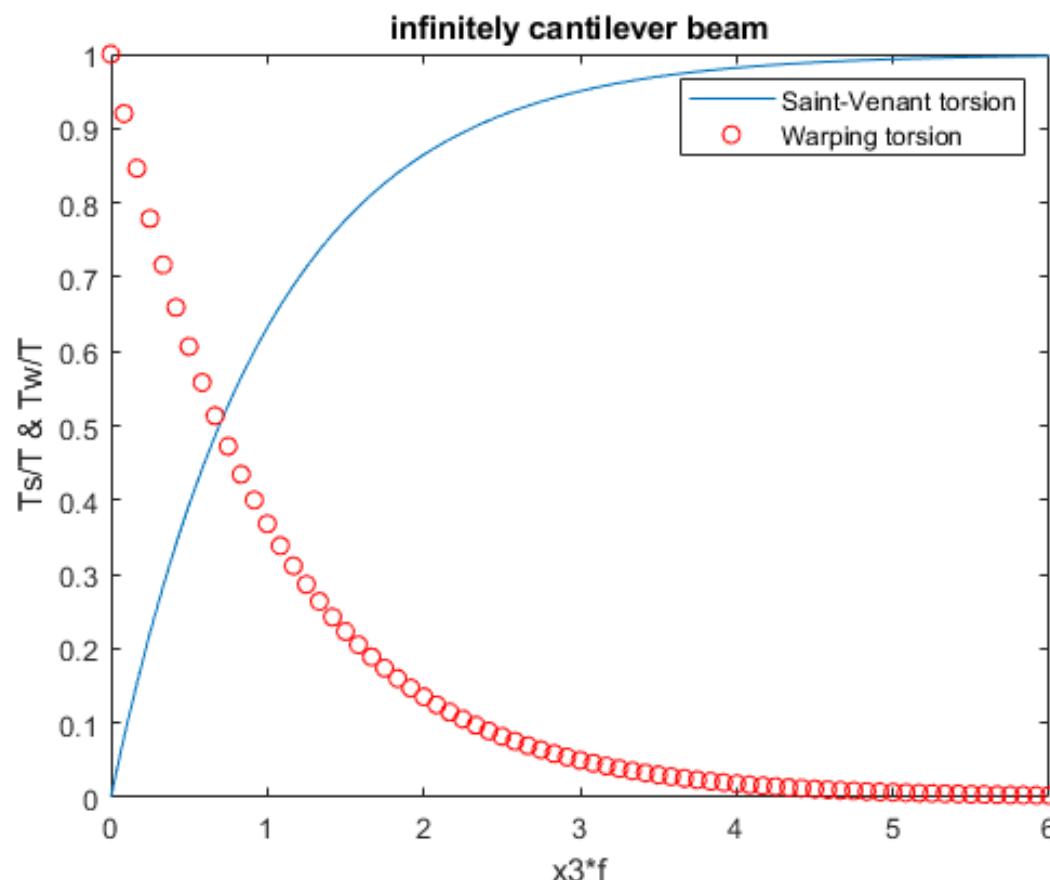
$$T_S = GK \phi'_3 = T_D (1 - e^{-x_3 f})$$

$$T_W = -(EI_{ww} \phi''_3)' = T_D (e^{-x_3 f})$$

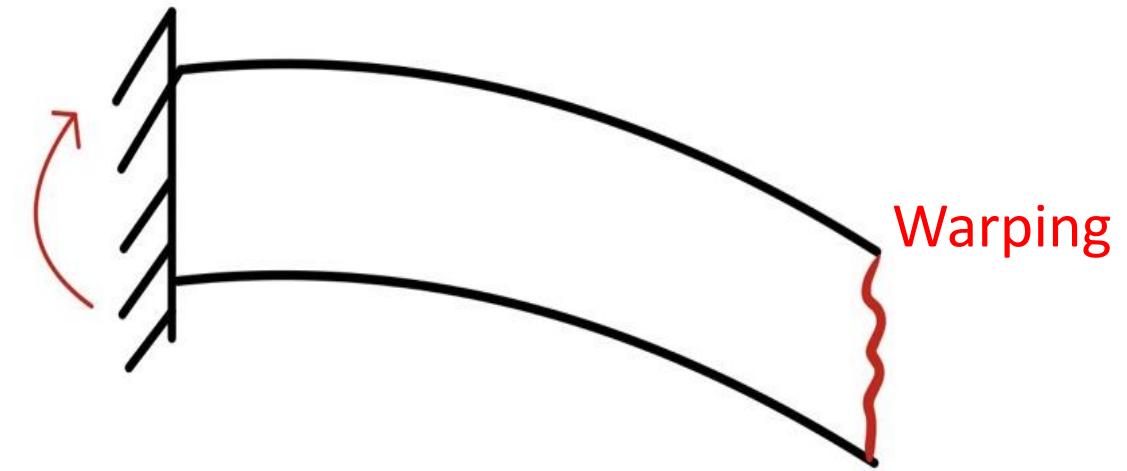


Warping torsion theory

Saint-Venant torsion vs. warping torsion



Warping moment



The warping torsion theory can display variations along the axial direction.
But it cannot display stresses and warping displacements on cross sections.

Methodology

1. According to the cross section, the stress function S is assumed.

$$-\nabla^2 S = \left[\frac{\nu E}{1+\nu} \kappa'_1 x_1 + \frac{df_2}{dx_1} \right] + \left[\frac{\nu E}{1+\nu} \kappa'_2 x_2 + \frac{df_1}{dx_2} \right] + 2G\phi'_3$$

Getting the relationship between stress function S & curvature κ'_1, κ'_2 & rate of twist per unit length ϕ'_3 .

3. Integrating the stress function S which equals T . Using the $T_S = GK\phi'_3$ get the cross section factor K .

4. Using Euler-Bernoulli beam formula .

$$T_w = -(EI_{ww} \phi''_3)'$$

5. Using formula $-EI_{ww}\theta'' + GK\theta = T$
to get $f = \sqrt{\frac{Gk}{EI_{ww}}} \quad \theta'' - f^2\theta = \frac{-T}{EI_{ww}}$

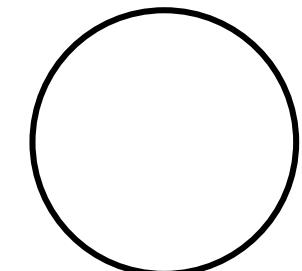
6. Getting the $\theta(x_3)$ & $\phi_3(x_3)$ according to the boundary condition

7. Using $EI_{ii}h_i \frac{d\theta}{dx_3} = M_i, i = 1, 2$
Getting normal stress σ_{33}

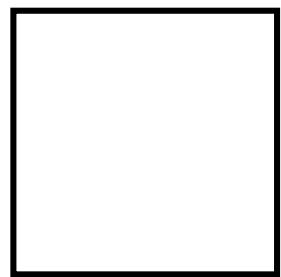
8. Getting stress function S & warping function W

Different cross sections

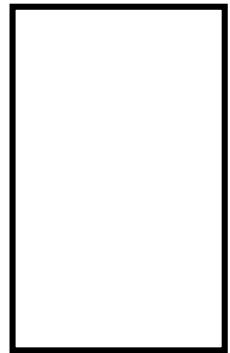
(e.g., ellipse, rectangle, thin-walled, singly-connected, multi-connected)



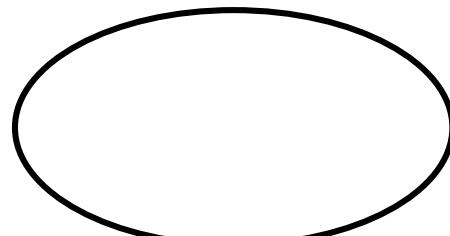
Circle



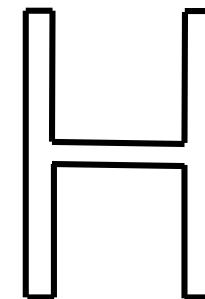
Square



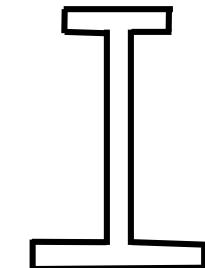
Rectangle



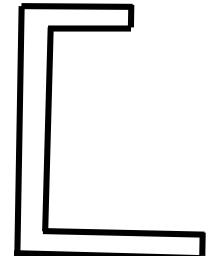
Ellipse



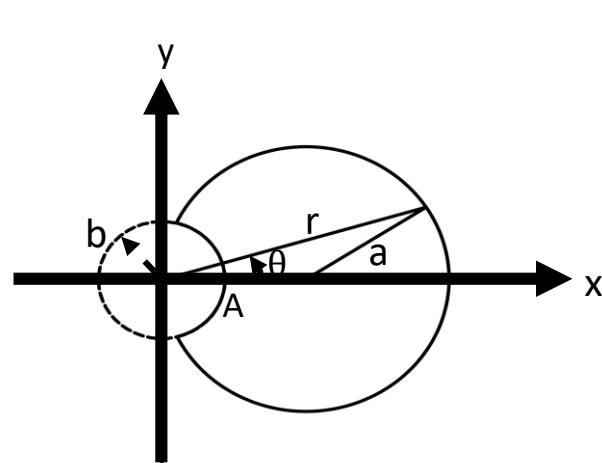
H-Section



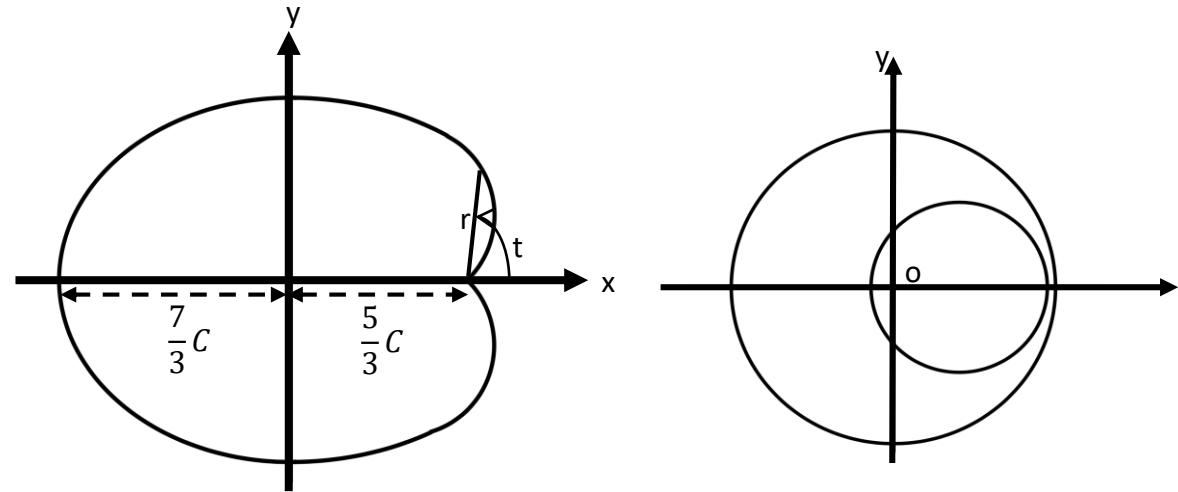
I-Section



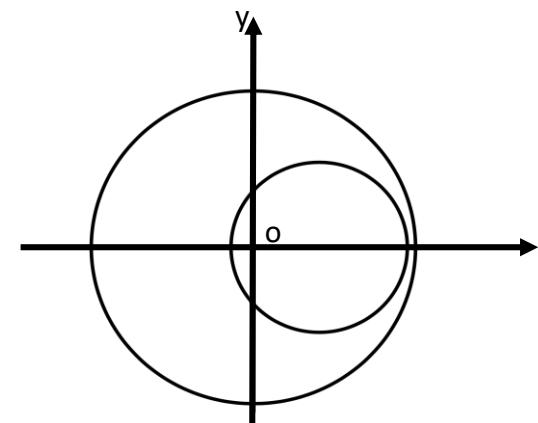
Channel



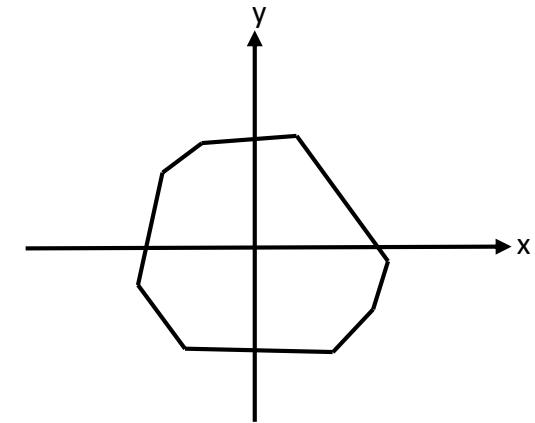
Keyway



Cardioid



Eccentric circles



Polygon

Different load types

$F_1 \& F_2 \& T$

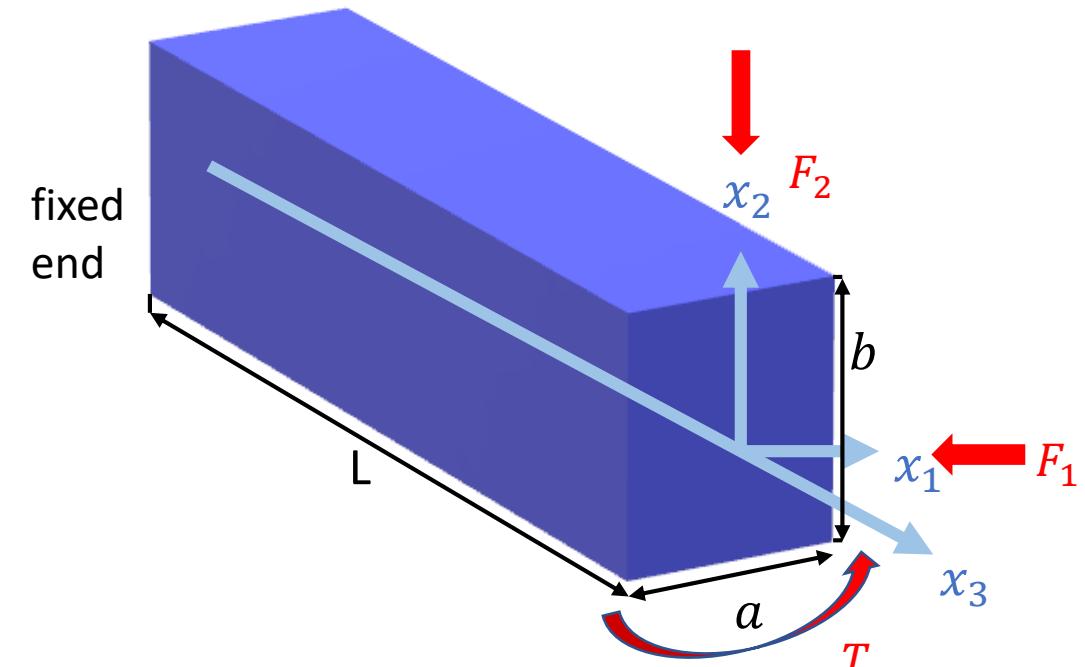
$$S_1 = \frac{\nu}{1+\nu} \frac{F_2}{6I_{11}} \left(\frac{a^2}{4} x_1 - x_1^3 \right) + \sum_{n=1}^{\infty} \frac{3a^3}{2(n\pi)^3} \frac{(-1)^n}{\cosh(\frac{n\pi b}{a})} \sin\left(\frac{2n\pi x_1}{a}\right) \cosh\left(\frac{2n\pi x_2}{a}\right)$$

$$S_2 = \frac{\nu}{1+\nu} \frac{F_1}{6I_{22}} \left(\frac{b^2}{4} x_2 - x_2^3 \right) + \sum_{n=1}^{\infty} \frac{3b^3}{2(n\pi)^3} \frac{(-1)^n}{\cosh(\frac{n\pi a}{b})} \sin\left(\frac{2n\pi x_2}{b}\right) \cosh\left(\frac{2n\pi x_1}{b}\right)$$

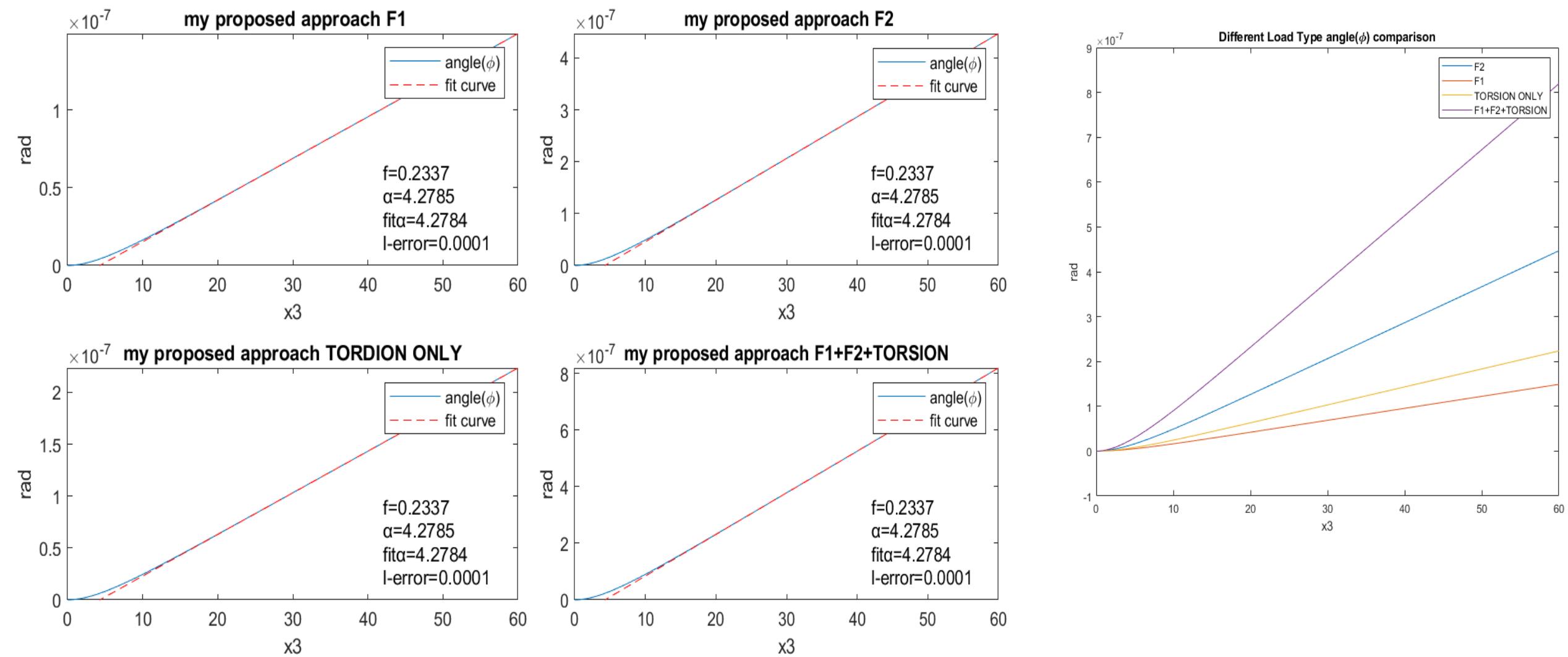
$$S_3 = G\phi'_3 \left(\frac{a^2}{4} - x_1^2 \right) + G\phi'_3 v(x_1, x_2)$$

$$v(x_1, x_2) = \sum_{n=1}^{\infty} A_n \cos(\lambda_n x_1) \cosh(\lambda_n x_2)$$

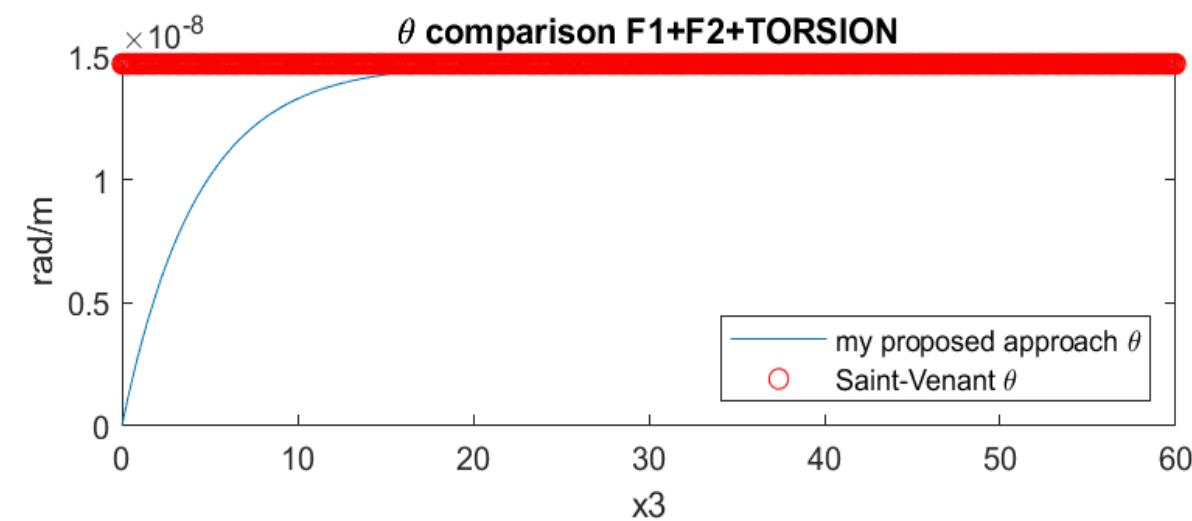
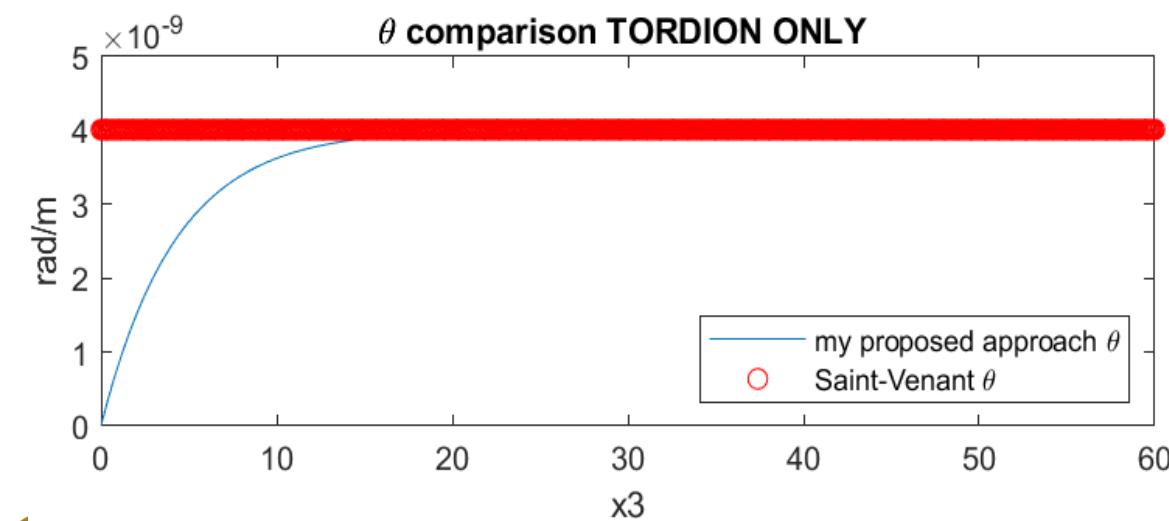
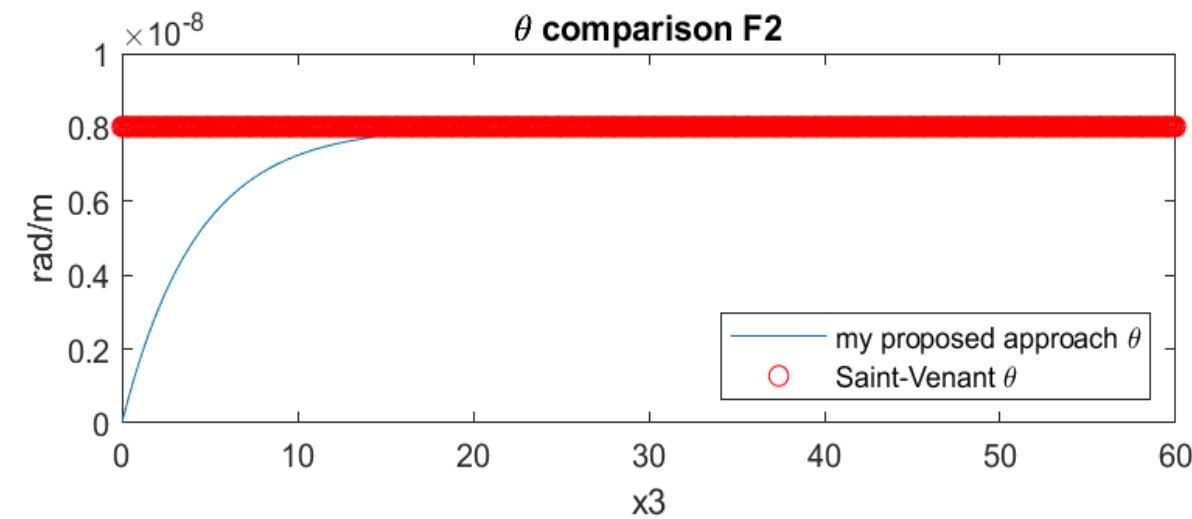
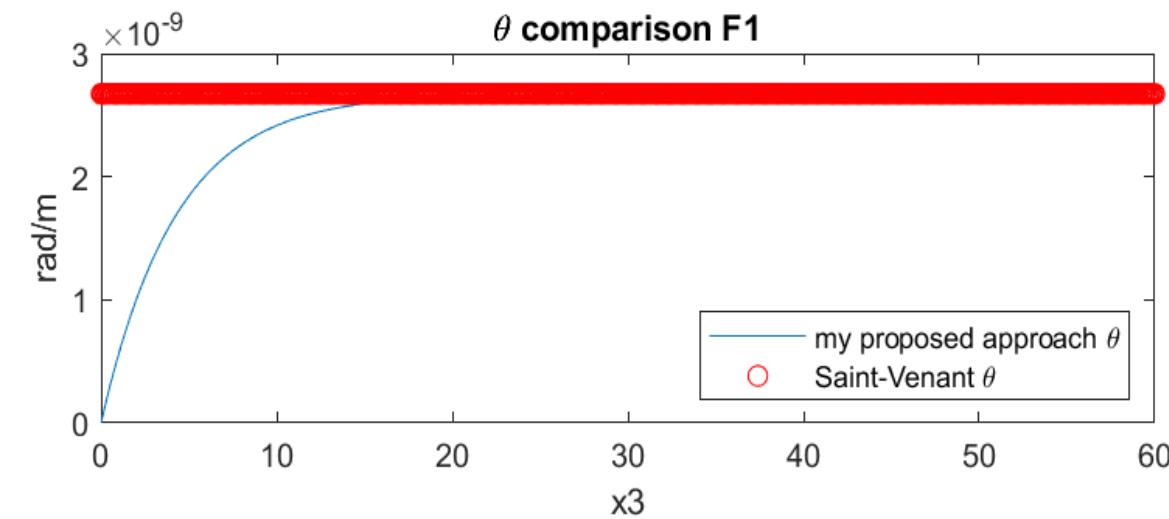
$$\lambda_n = \frac{(2n-1)\pi}{a} \quad A_n = \frac{8a^2(-1)^n}{(2n-1)^3\pi^3 \cosh(\frac{\lambda_n b}{2})}$$



Verification-rectangular F_1 & F_2 &T

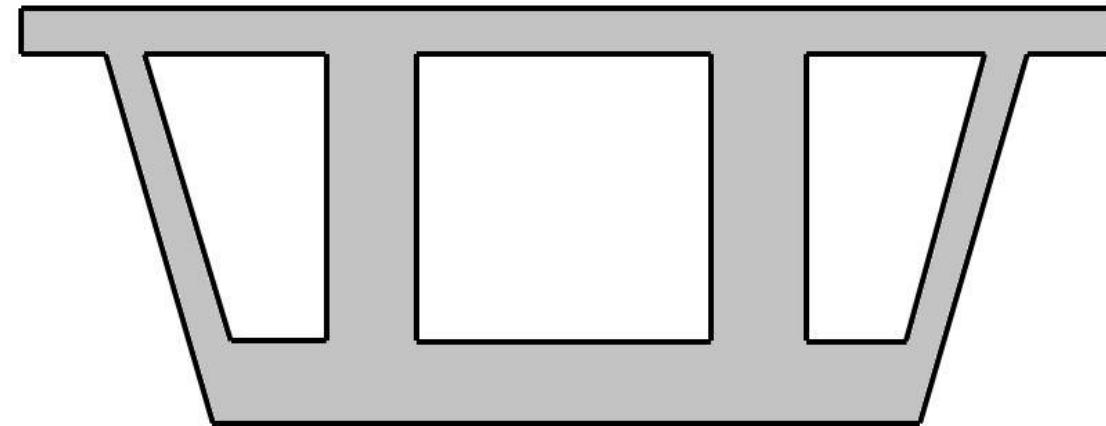


Verification-rectangular F_1 & F_2 &T



Thin-walled cross section

Thin-walled multi-cells



Thin-walled cross section

$$T_S = GK\phi'_3 \quad T_w = -(EI_{ww} \phi''_3)'$$



Thin-walled cross section

$$T_S = GK\phi'_3 \quad T_w = -(EI_{ww} \phi''_3)'$$

According to the cross section, the stress function *S* is assumed.

Assuming stress
function directly.

Using $v(x_1, x_2)$ to get the
stress function.

Using shear flow

Shear center (x_1^{sc}, x_2^{sc})

$$x_1^{sc} = \frac{I_{22}J_1 - I_{12}J_2}{2(1+v)(I_{11}I_{22} - I_{12}^2)},$$
$$x_2^{sc} = \frac{I_{11}J_2 - I_{12}J_1}{2(1+v)(I_{11}I_{22} - I_{12}^2)}$$

where

$$J_1 = \int_R \left[\nu x_1^3 - (1 + \nu)x_1 x_2^2 - x_2 \frac{\partial \mathcal{W}_1}{\partial x_1} + x_1 \frac{\partial \mathcal{W}_1}{\partial x_2} \right] dA,$$

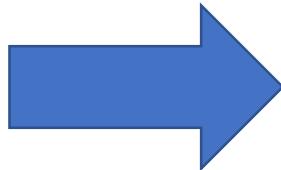
$$J_2 = \int_R \left[\nu x_2^3 - (1 + \nu)x_1^2 x_2 + x_1 \frac{\partial \mathcal{W}_2}{\partial x_2} - x_2 \frac{\partial \mathcal{W}_2}{\partial x_1} \right] dA$$

$$\kappa'_\alpha = \frac{1}{E} (I^{-1})_{\alpha\beta} e_{3\beta\gamma} P_\gamma$$



$$\sigma_{ij}(x_1, x_2, x_3) = \begin{bmatrix} 0 & 0 & \frac{\partial S}{\partial x_2} + c_2 \frac{x_1^2}{2} + f_1(x_2) \\ 0 & 0 & -\frac{\partial S}{\partial x_1} - c_2 \frac{x_2^2}{2} + f_2(x_2) \\ \text{sym} & \text{sym} & \sigma_{33} \end{bmatrix}$$

Elasticity



Elastoplasticity



Continuum mechanics material model of flow elastoplasticity

Saint-Venant flow plasticity vs. deformation plasticity

Prandtl-Reuss **flow elastoplastic** model with **von Mises** yield surface

Flow elastoplastic model with **cubic distortional** yield surface¹

- Active deviatoric stress closed-form solution: 閉合正解

$$\mathbf{s}_a^\alpha(t) = \left[\frac{\frac{\mathbf{s}_a^\alpha(t_1)}{R^\alpha(\lambda^\alpha(t_1))} + \frac{(a^\alpha - 1)}{\|\dot{\mathbf{s}}^\alpha\|^2} \dot{\mathbf{s}}^\alpha \dot{\mathbf{s}}^{\alpha T} \frac{\mathbf{s}_a^\alpha(t_1)}{R^\alpha(\lambda^\alpha(t_1))} + b^\alpha \frac{\dot{\mathbf{s}}^\alpha}{\|\dot{\mathbf{s}}^\alpha\|}}{b^\alpha \left(\frac{\dot{\mathbf{s}}^\alpha}{\|\dot{\mathbf{s}}^\alpha\|} \right)^T \frac{\mathbf{s}_a^\alpha(t_1)}{R^\alpha(\lambda^\alpha(t_1))} + a^\alpha} \right] R^\alpha(\lambda^\alpha(t))$$

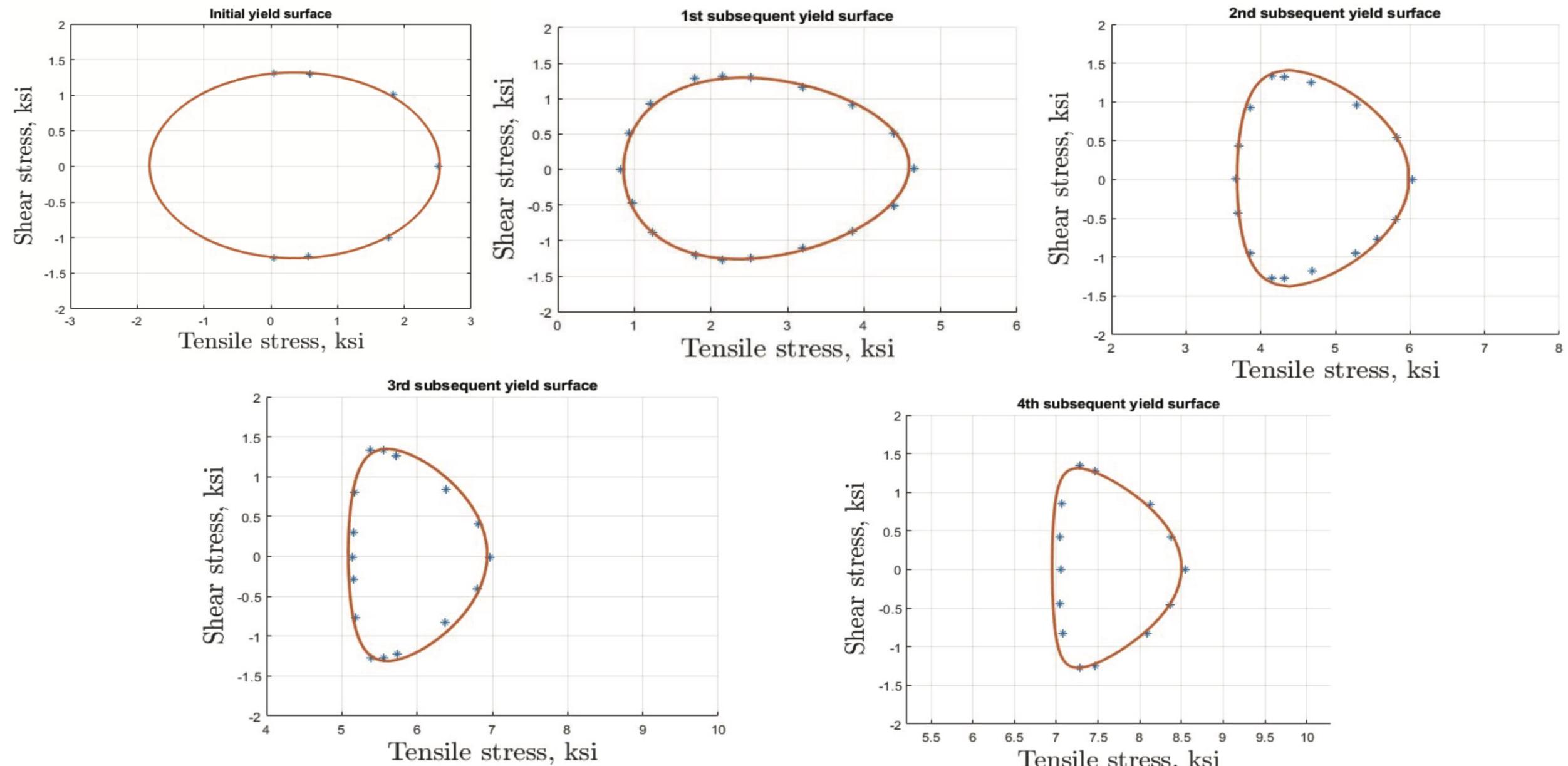
- Back deviatoric stress closed-form solution: 閉合正解

$$\mathbf{s}_b^\alpha(t) = \exp\left(\frac{k_p^\alpha}{\eta^\alpha} [\lambda^\alpha(t_1) - \lambda^\alpha(t)]\right) \mathbf{s}_b^\alpha(t_1) + \frac{\eta^\alpha}{R_\infty^\alpha} \left\{ \mathbf{s}_a^\alpha(t) - \exp\left(\frac{k_p^\alpha}{\eta^\alpha} [\lambda^\alpha(t_1) - \lambda^\alpha(t)]\right) \mathbf{s}_a^\alpha(t_1) \right\}$$

- Equivalent plastic strain closed-form solution: 閉合正解

$$\lambda^\alpha(t) = \frac{R^\alpha(\lambda^\alpha(t_1))}{R^\alpha(\lambda^\alpha(t))} \lambda^\alpha(t_1) + \frac{R_\infty^\alpha R^\alpha(0)}{k_p^\alpha R^\alpha(\lambda^\alpha(t))} \ln[b^\alpha \left(\frac{\dot{\mathbf{s}}^\alpha}{\|\dot{\mathbf{s}}^\alpha\|} \right)^T \frac{\mathbf{s}_a^\alpha(t_1)}{R^\alpha(\lambda^\alpha(t_1))} + a^\alpha]$$

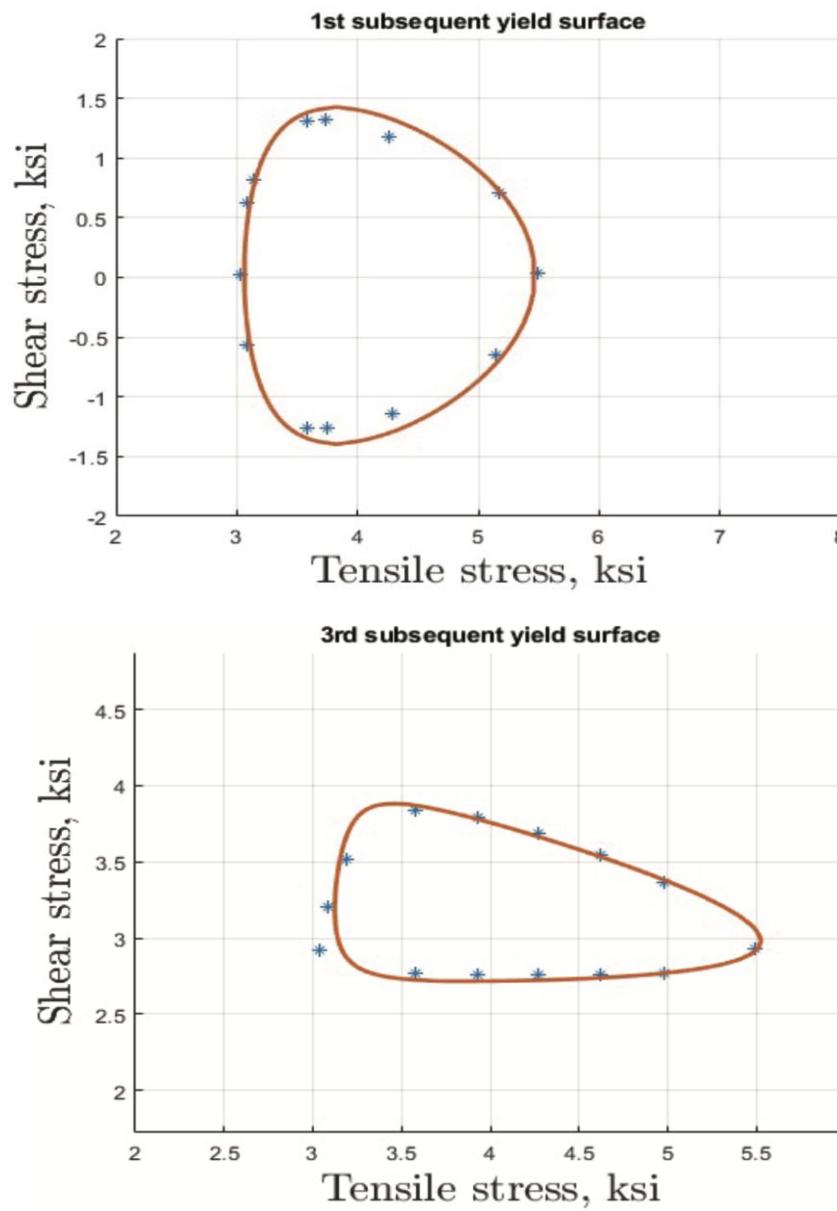
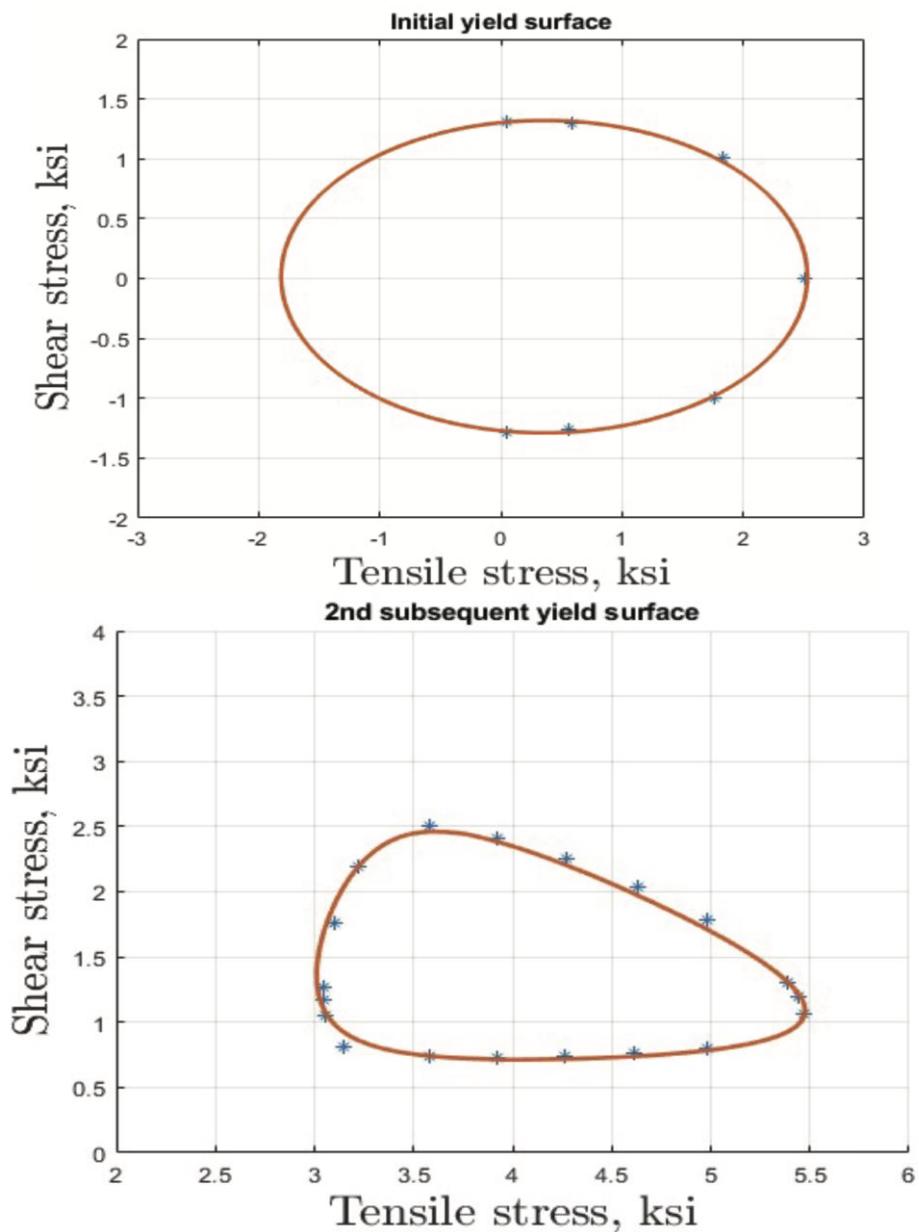
¹Hong, H. K., Liu, L. W., Shiao, Y. P., & Yan, S. F. (2022). Yield Surface Evolution and Elastoplastic Model with Cubic Distortional Yield Surface. *ASCE Journal of Engineering Mechanics*, 148(6), 04022027.



A. Phillips & J.-L. Tang, The effect of loading path on the yield surface at elevated temperatures. *International Journal of Solids and Structures*, 8(4) 463-474, 1972.

K.-M. Hou, The evolution of cubic distortional yield hypersurfaces in materials of flow elastoplasticity under prestress and at elevated temperatures. MS Thesis, Civil Engrg. Dept., National Taiwan University, 2023.





A. Phillips & J.-L. Tang, The effect of loading path on the yield surface at elevated temperatures. *International Journal of Solids and Structures*, 8(4) 463-474, 1972.

K.-M. Hou, The evolution of cubic distortional yield hypersurfaces in materials of flow elastoplasticity under prestress and at elevated temperatures. MS Thesis, Civil Engrg. Dept., National Taiwan University, 2023.

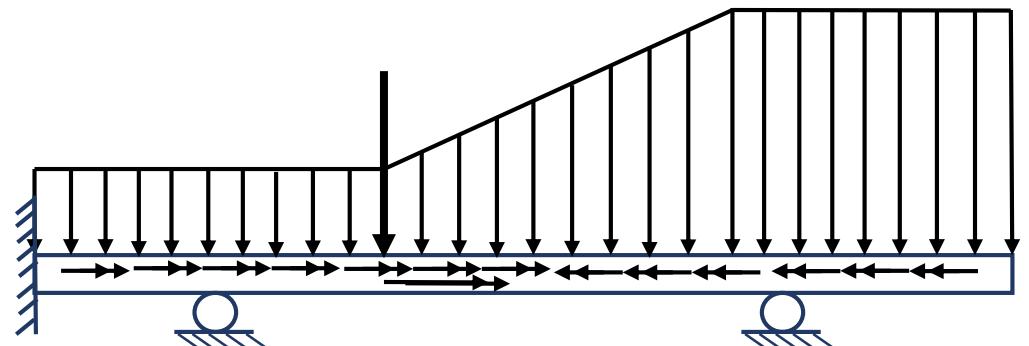


Structural model of flow elastoplasticity

Saint-Venant's "relaxed" conditions prescribing the stress resultants :

$$\left. \begin{array}{l} \int_R \sigma_{13} dA = N_1(x_3) = F_1, \\ \int_R \sigma_{23} dA = N_2(x_3) = F_2, \\ \int_R \sigma_{33} dA = N_3(x_3) = 0, \\ \int_R x_2 \sigma_{33} dA = M_1(x_3) = -(L - x_3)F_2, \\ - \int_R x_1 \sigma_{33} dA = M_2(x_3) = (L - x_3)F_1, \\ \int_R (x_1 \sigma_{23} - x_2 \sigma_{13}) dA = T(x_3) = x_1^P F_2 - x_2^P F_1, \end{array} \right\}$$

Statically indeterminate

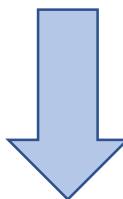


on the cross section planes $x_3 = 0, L$, with loads F_1, F_2 and point of application (x_1^p, x_2^p) prescribed.

CM material model of flow elastoplasticity

Boundary/interior support condition

Force/moment load type



Saint-Venant's “relaxed” condition

Structural model of flow elastoplasticity

i.e. Generalized plastic hinge

Conclusions

1. Computing **stresses and warping displacements** on cross sections and also their variations along the axial direction, *i.e.*, in **three dimensions**.
2. Valid for **various prismatic cross sections** (circle, ellipse, rectangle, polygon, I-section, H-section, channel, thin-walled multi-cells, cardioid, eccentric circles, grooved keyway, multi-connected). Formulae for cross section factor K , warping factor f , shear center location.
3. Valid for **different loading types** (concentrated and distributed force & moment loads) and different boundary/interior conditions on c_1, c_2, c_3, c_4 .
4. Elasticity to continuum mechanics material & structural models of flow elastoplasticity.

Acknowledgment: H.-H. Huang, Three-dimensional Saint-Venant flexure-torsional and warping. MS Thesis, Civil Engrg. Dept., National Taiwan University, 2023.
National Science and Technology Council of Taiwan under Grant MOST 111-2221-E-002-055-MY2.

THANKS





國立臺灣大學
National Taiwan University

FISV

ACMT

WCCM-PANACM
VANCOUVER 2024

Methodology-different load types



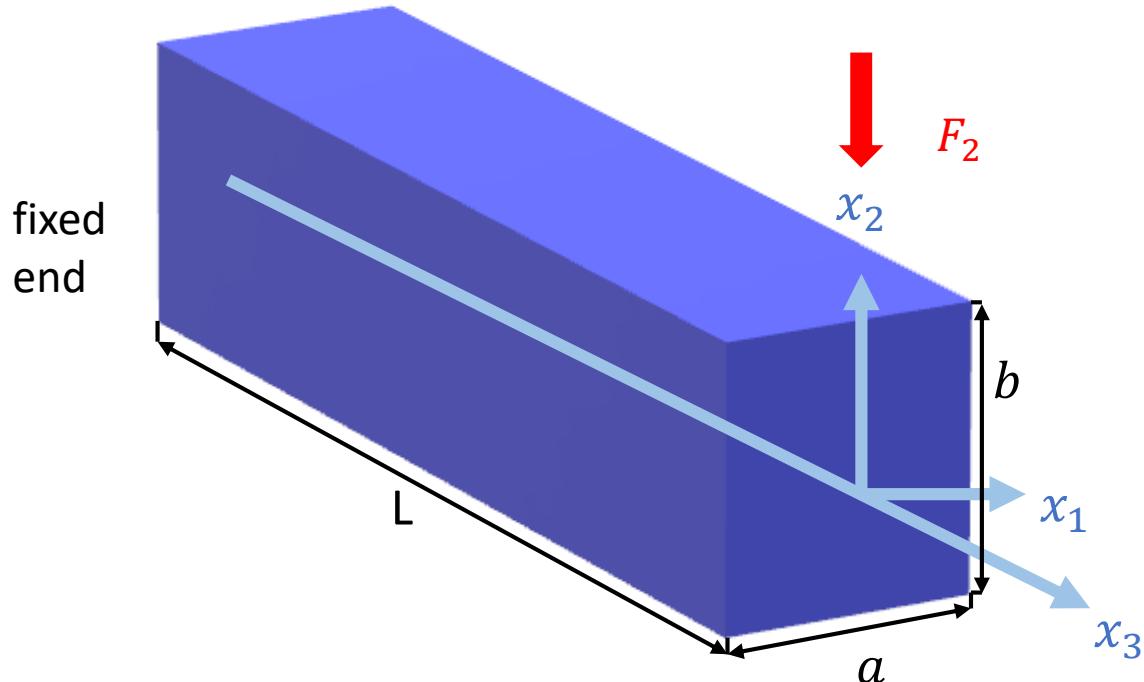
國立臺灣大學
National Taiwan University

FISV

ACMT

WCCM-PANACM
VANCOUVER 2024

Methodology-different loading types



$$\text{GE} \quad -\nabla^2 S = \left[\frac{\nu E}{1+\nu} \kappa'_1 x_1 + \frac{df_2}{dx_1} \right] + \left[\frac{\nu E}{1+\nu} \kappa'_2 x_2 + \frac{df_1}{dx_2} \right] + 2G\phi'_3$$
$$\text{BC} \quad \frac{ds}{ds} = \frac{dx_1}{ds} \left[-E\kappa'_1 \frac{x_2^2}{2} + f_2(x_1) \right] - \frac{dx_2}{ds} \left[E\kappa'_2 \frac{x_1^2}{2} + f_1(x_2) \right]$$

$$\kappa'_1 = \frac{c_1}{E} = \frac{M'_1}{EI_{11}} = \frac{F_2}{I_{11}}$$

When $x_1 = \frac{a}{2}$,

$$\frac{\partial S}{\partial s} = 0, \quad -E\kappa'_1 \frac{x_2^2}{2} + f_2(x_1) = 0, \quad f_2(x_1) = \frac{Fb^2}{8I_{11}}$$

$$\begin{cases} -\nabla^2 S_1 = \left[\frac{\nu E}{1+\nu} + \frac{F_2 x_1}{I_{11}} \right] \\ S_1 = 0 \end{cases}$$

Methodology- F_2

When $x_1 = \frac{a}{2}$, $\begin{cases} -\nabla^2 S_1 = \left[\frac{\nu E}{1+\nu} + \frac{F_2 x_1}{I_{11}} \right] \\ S_1 = 0 \end{cases}$

$$H = S - P$$

$$P = A_1 x_1^3 + A_2 x_1^2 + A_3 x_1 + A_4$$

$$A_1 = \frac{\nu}{1+\nu} \frac{F_2}{6I_{11}}, \quad A_2 = 0$$

$$H = S - P = S - \frac{\nu}{1+\nu} \frac{F_2}{6I_{11}} x_1^3 + A_3 x_1 + A_4$$

$$\begin{cases} -\nabla^2 H_1 = 0 \\ H_1 = \frac{\nu}{1+\nu} \frac{F_2}{6I_{11}} x_1^3 + A_3 x_1 + A_4 \end{cases}$$

$$A_3 = \frac{\nu}{1+\nu} \frac{6}{I_{11}} \frac{a^2}{4}, \quad A_4 = 0$$

$$\begin{cases} \nabla^2 H_1 = 0 & (R) \\ H_1 = 0 & \text{for } x_1 = \pm \frac{a}{2} \\ H = \frac{\nu}{1+\nu} \frac{F_2}{6I_{11}} (x_1^3 - \frac{a^2}{4} x_1) & \text{for } x_2 = \pm \frac{b}{2} \end{cases}$$

$$\begin{aligned} H(x_1, x_2) &= \sum_{n=1}^{\infty} X(x_1) Y(x_2) \\ &= \sum_{n=1}^{\infty} B_n \sin\left(\frac{2n\pi x_1}{a}\right) \cosh\left(\frac{2n\pi x_2}{a}\right) \end{aligned}$$



Methodology- F_2

$$B_n = \frac{\nu}{1+\nu} \frac{F_2}{6I_{11}} \frac{(-1)^n}{\cosh\left(\frac{n\pi b}{a}\right)} \frac{3a^3}{2(n\pi)^3}$$

$$S_1 = \frac{\nu}{1+\nu} \frac{F_2}{6I_{11}} \left(\frac{a^2}{4} x_1 - x_1^3 \right) + \sum_{n=1}^{\infty} \frac{3a^3}{2(n\pi)^3} \frac{(-1)^n}{\cosh\left(\frac{n\pi b}{a}\right)} \sin\left(\frac{2n\pi x_1}{a}\right) \cosh\left(\frac{2n\pi x_2}{a}\right)$$

$$S_2 = \frac{\nu}{1+\nu} \frac{F_1}{6I_{22}} \left(\frac{b^2}{4} x_2 - x_2^3 \right) + \sum_{n=1}^{\infty} \frac{3b^3}{2(n\pi)^3} \frac{(-1)^n}{\cosh\left(\frac{n\pi a}{b}\right)} \sin\left(\frac{2n\pi x_2}{b}\right) \cosh\left(\frac{2n\pi x_1}{b}\right)$$



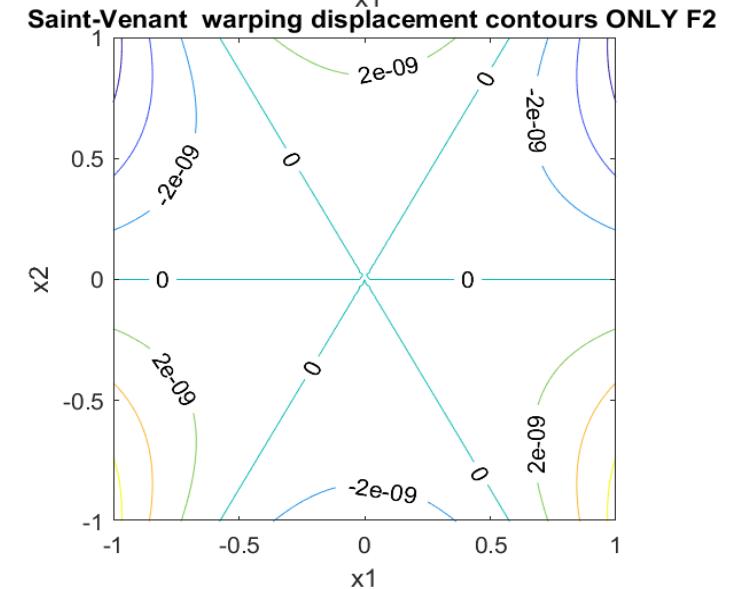
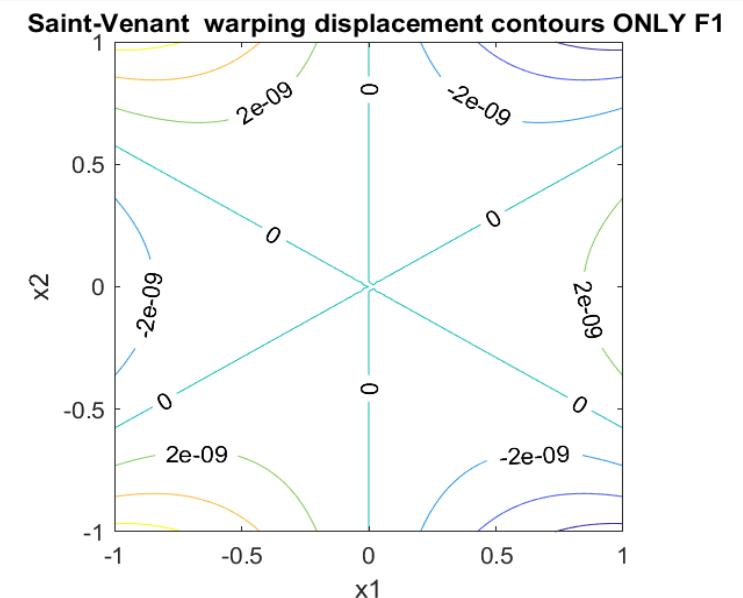
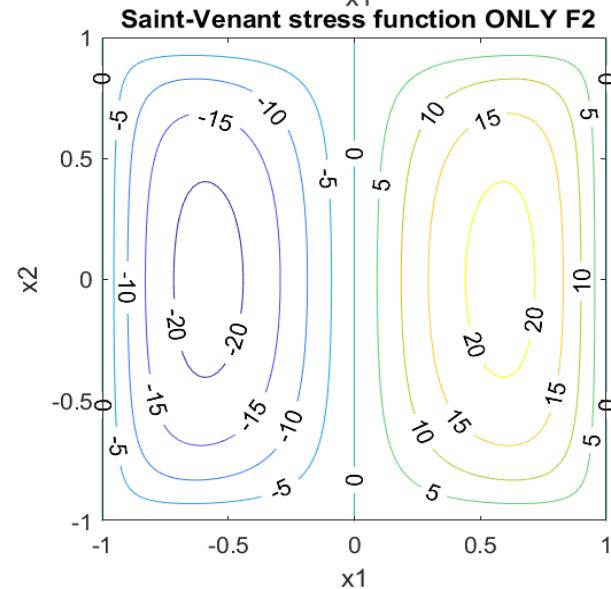
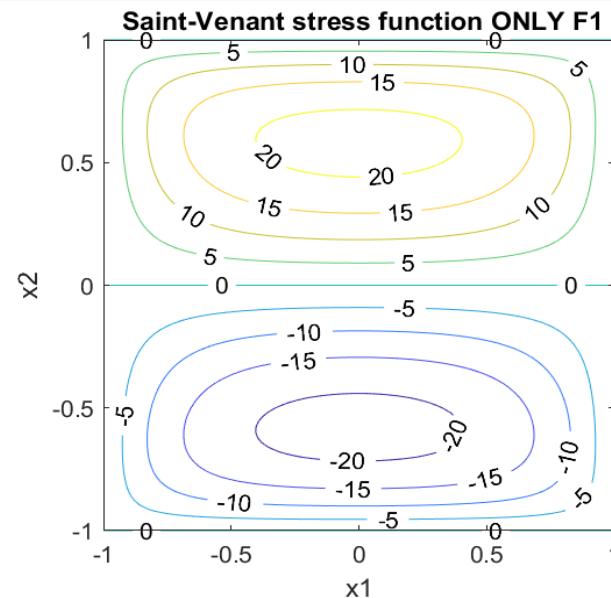
Comparison- F_1 & F_2

Comparison- F_1 & F_2

$$a = 2 \text{ m}, \quad b = 2 \text{ m}$$

$$F_1 = 2000 \text{ N}, \quad F_2 = 2000 \text{ N}$$

$$xp_1 = 0, xp_2 = 0$$

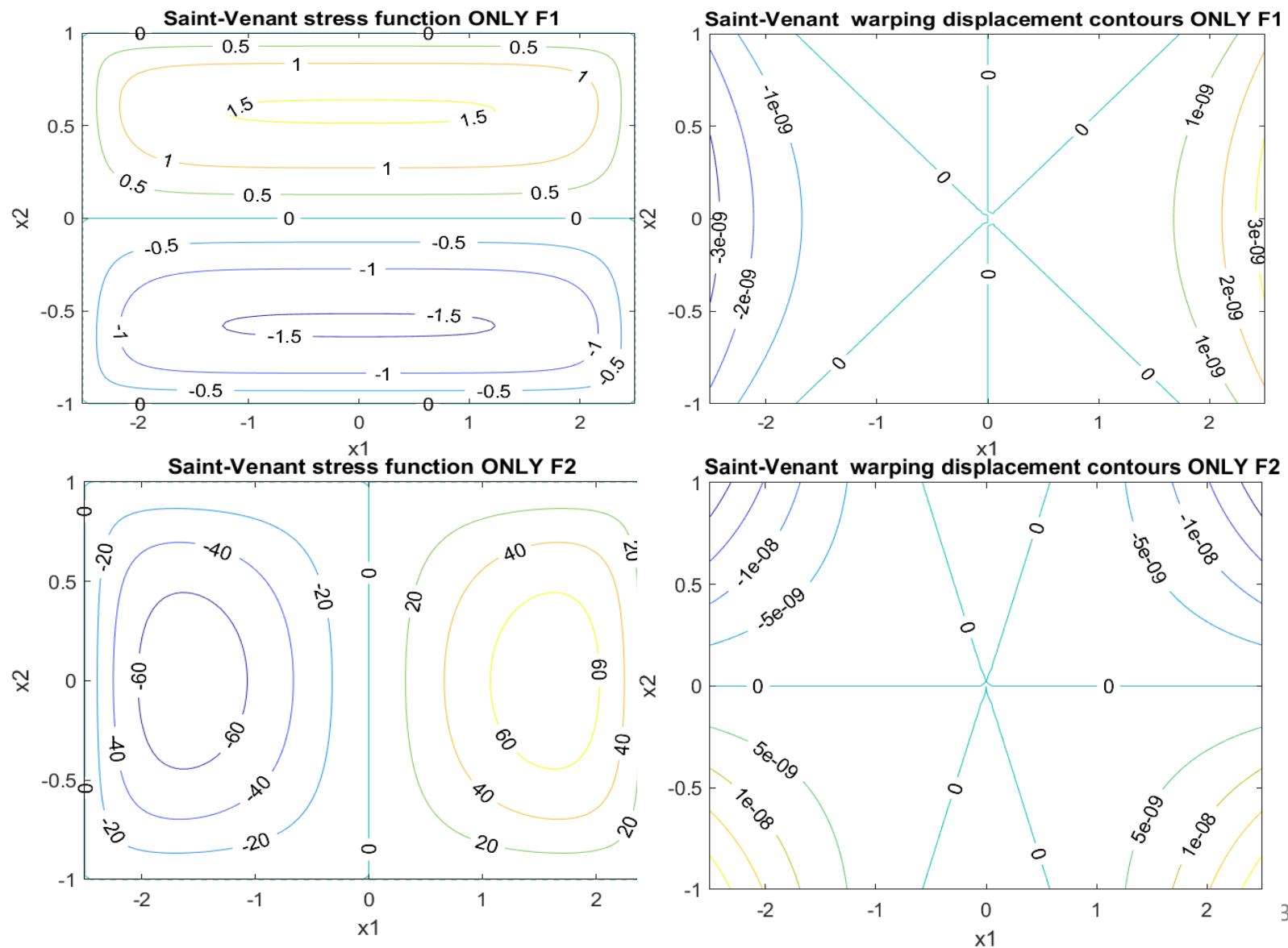


Comparison- F_1 & F_2

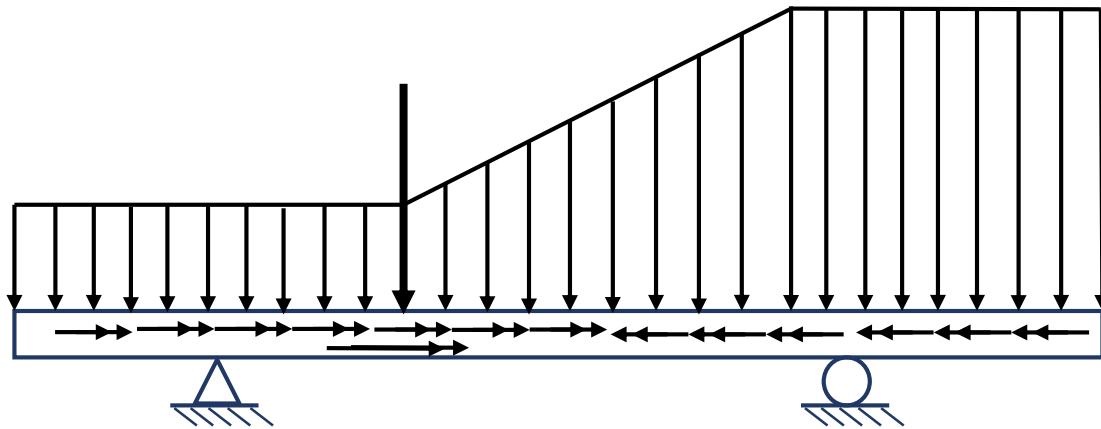
$a = 5 \text{ m}, \quad b = 2 \text{ m}$

$F_1 = 2000 \text{ N}, \quad F_2 = 2000 \text{ N}$

$xp_1 = 0, xp_2 = 0$



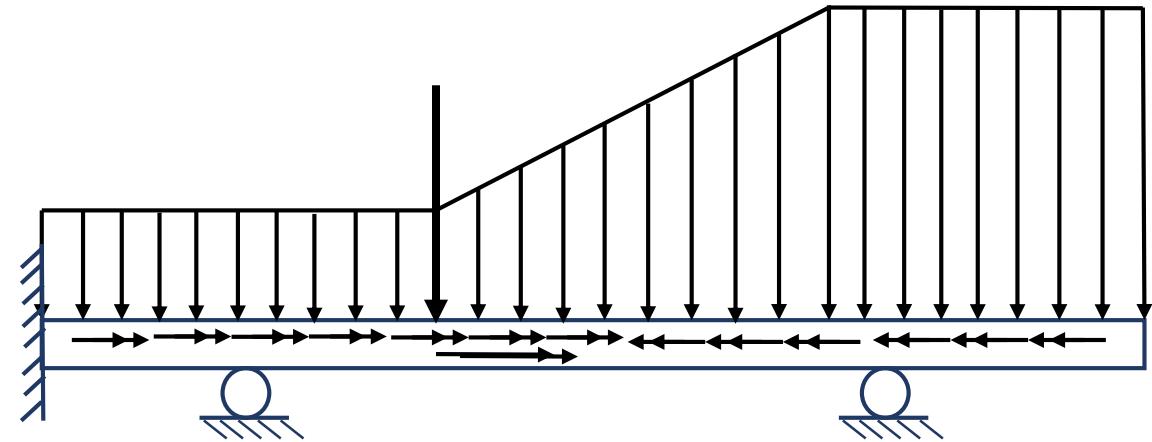
Statically determinate



↓↓↓↓ Distributed loads

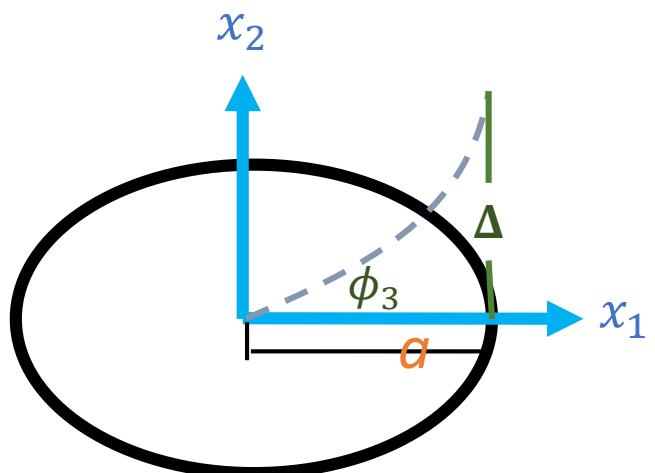
↓ Concentrated loads

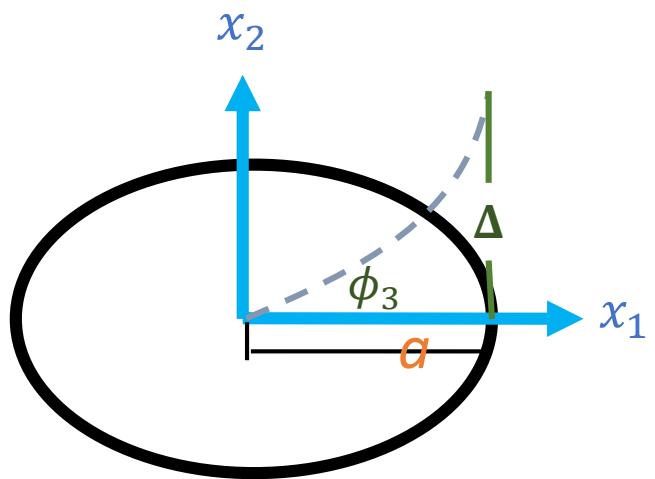
Statically indeterminate



→→→→ Distributed torsional loads

→→ Concentrated torsional loads





n_i

$$n_i = \begin{bmatrix} n_1 \\ n_2 \\ 0 \end{bmatrix} = \begin{bmatrix} \cos(n, x_1) \\ \cos(n, x_2) \\ 0 \end{bmatrix} = \begin{bmatrix} \frac{dx_1}{dn} \\ \frac{dx_1}{ds} \\ -\frac{dx_1}{ds} \end{bmatrix}$$



National Taiwan University



WCCM-EANACM
VANCOUVER 2024

I_{ww}

$I_{ww} = \int w^2 dF$: sectorial moment of inertia

w : displacement in z-direction



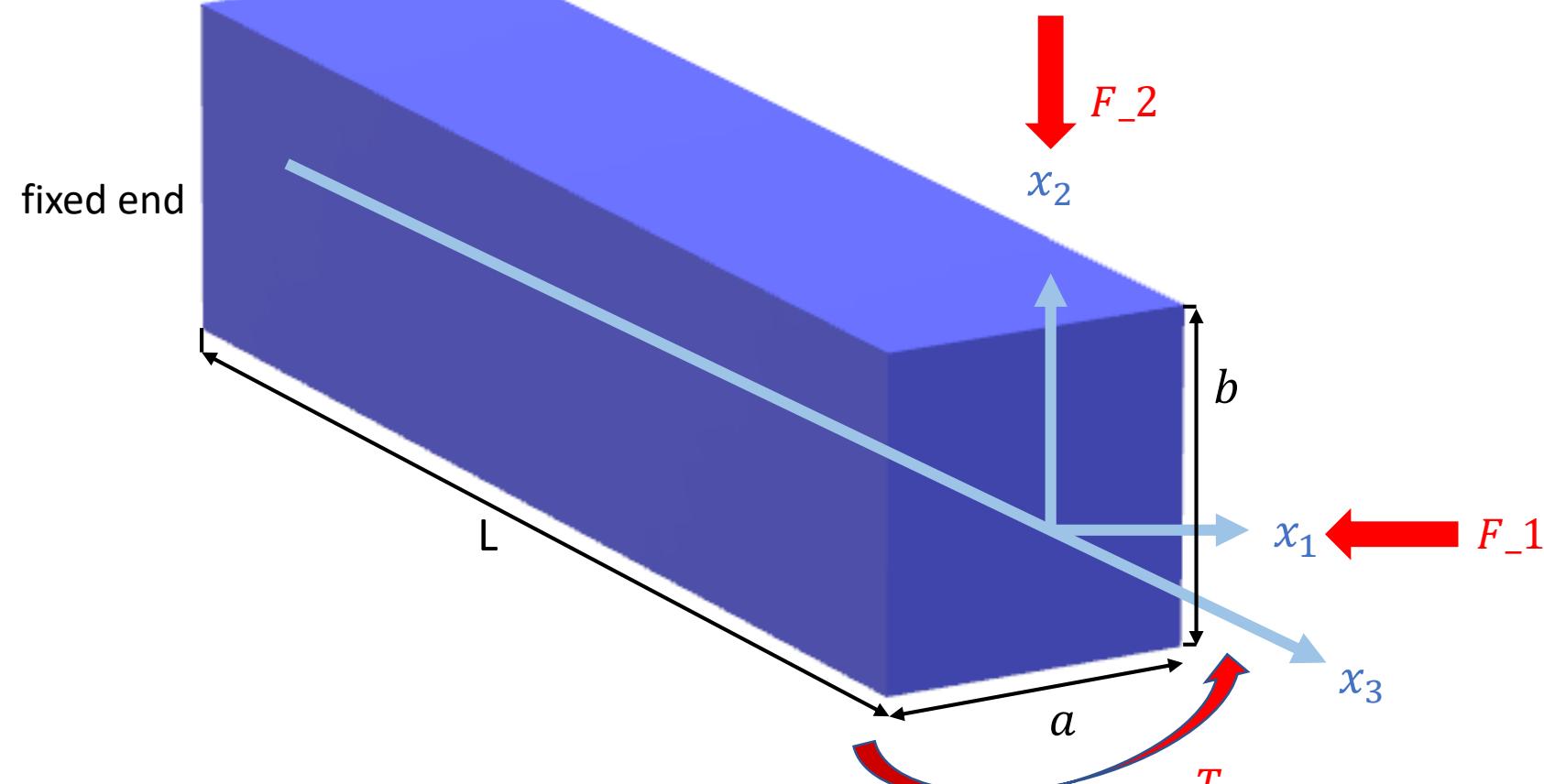
臺灣大學
National Taiwan University

HSV

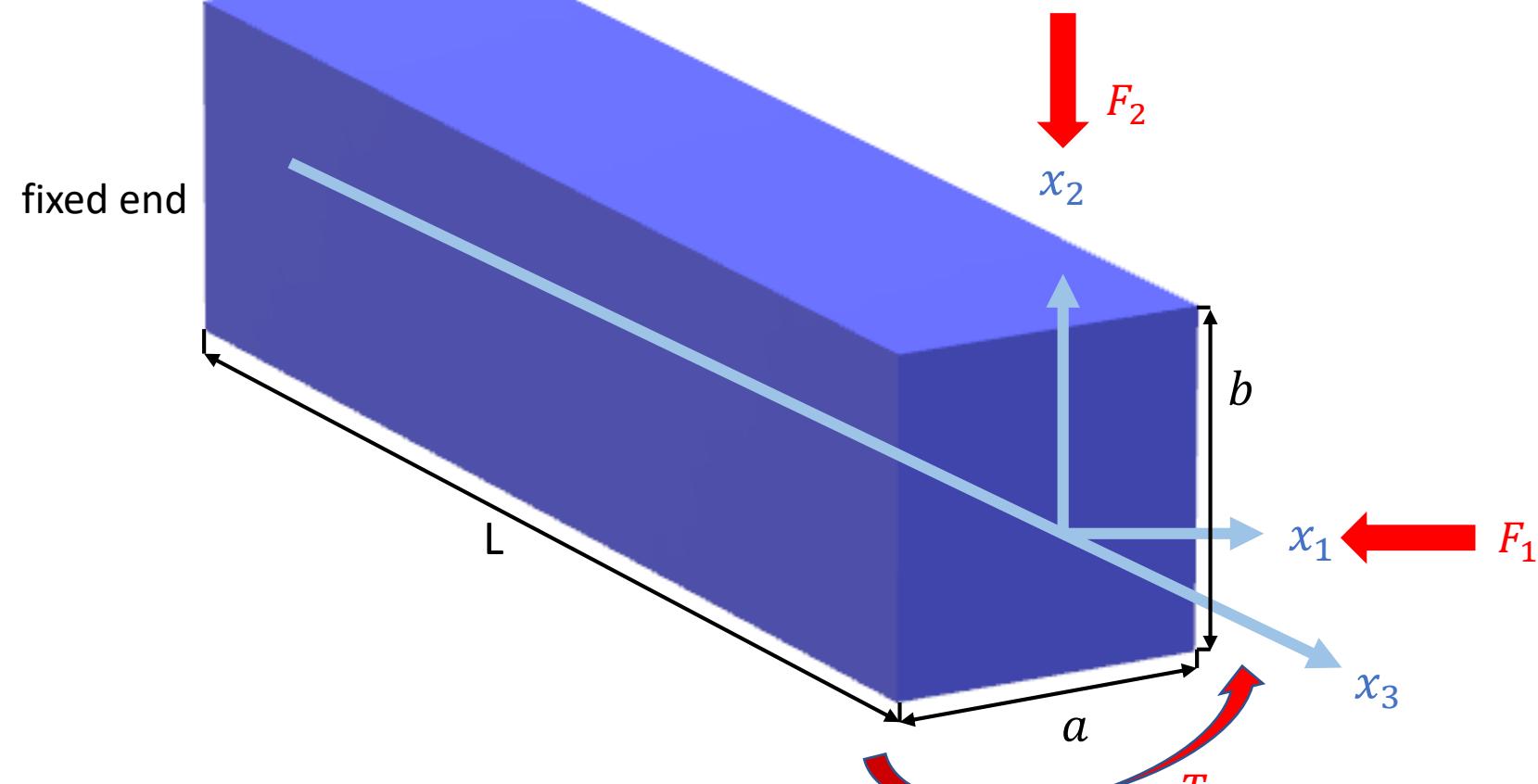
ACMTE

WCCM-PANACM
VANCOUVER 2024

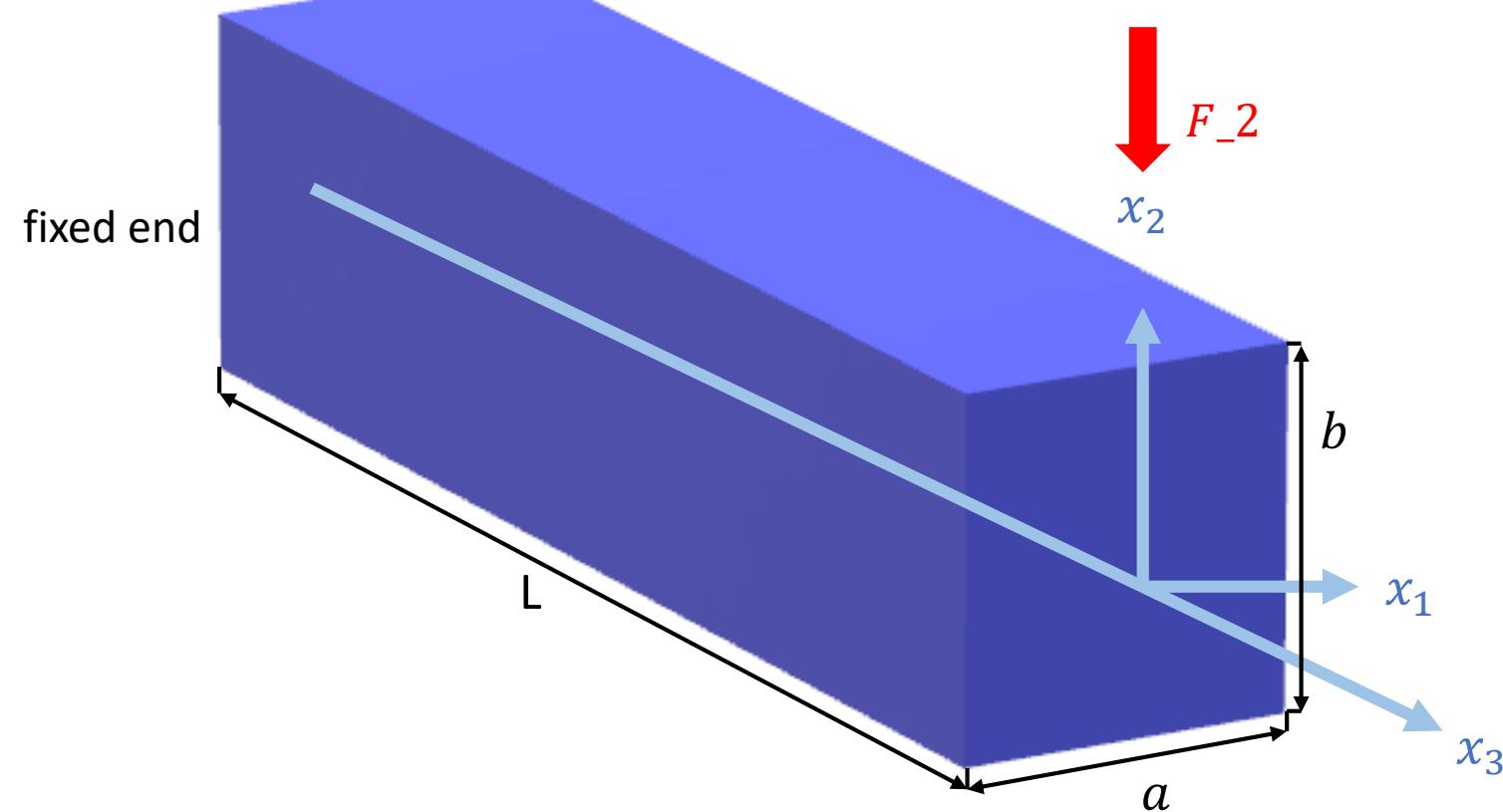
Recent research



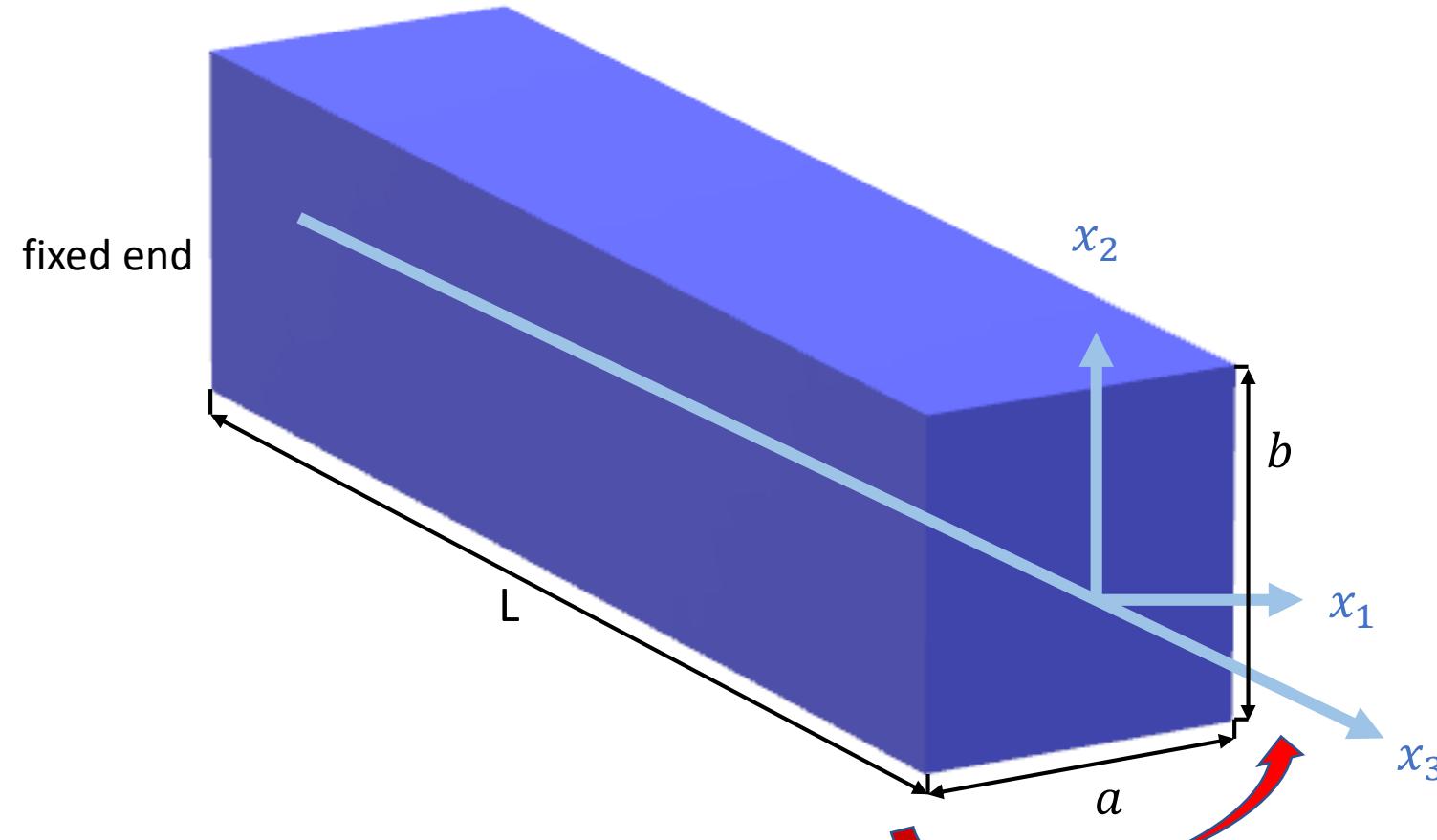
Recent research



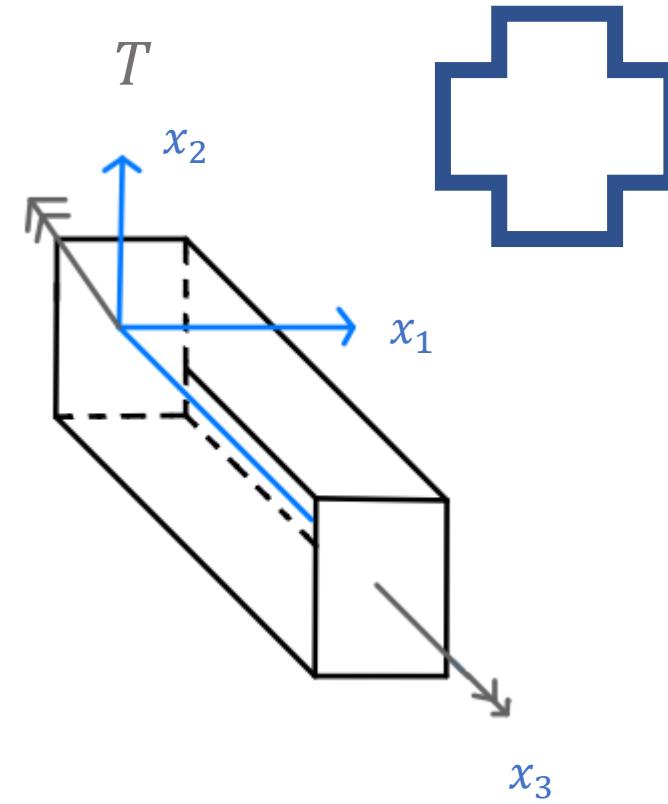
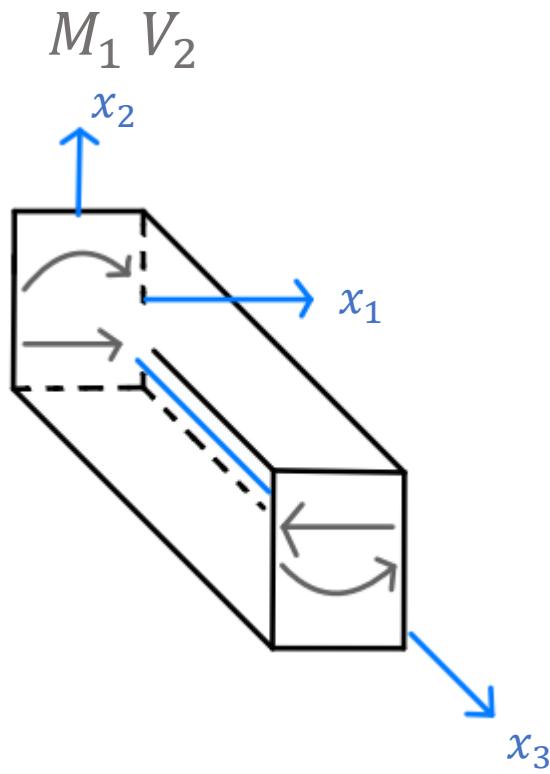
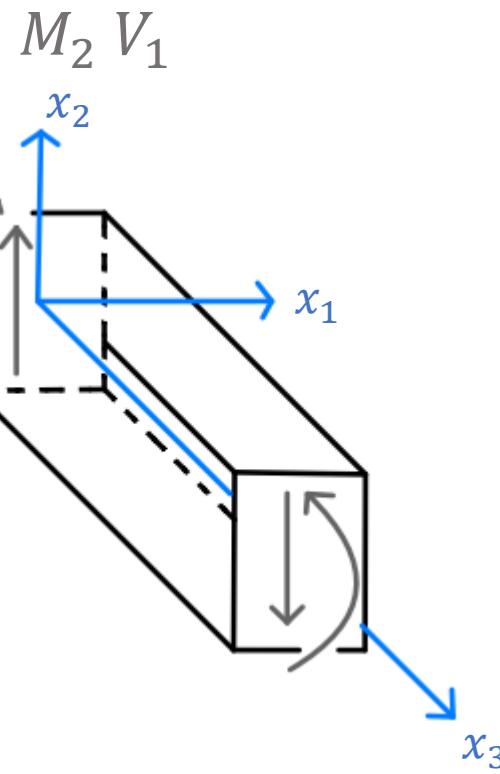
Recent research



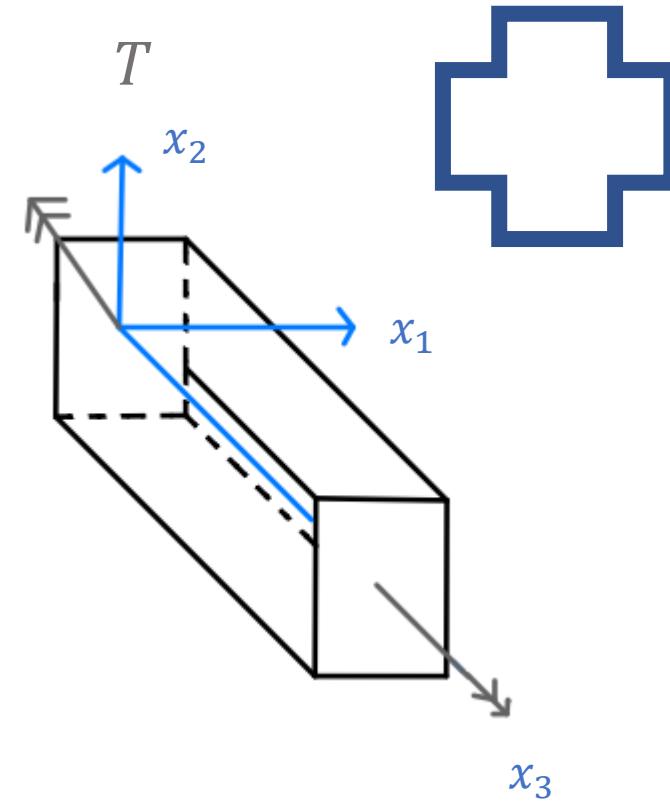
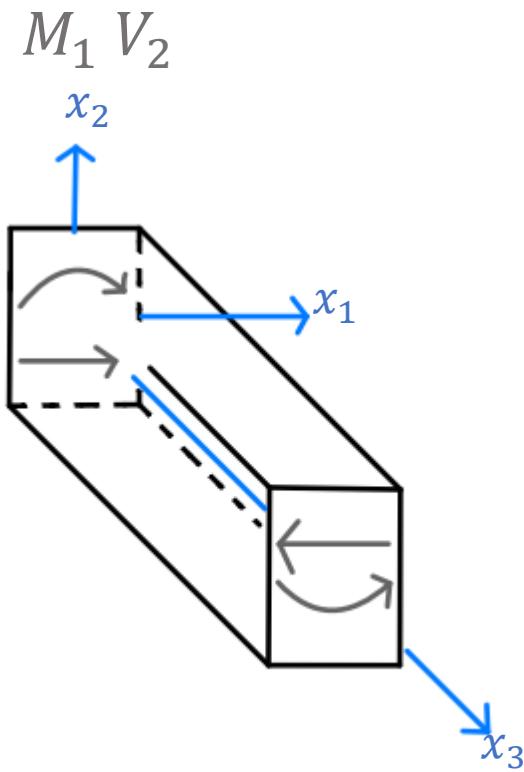
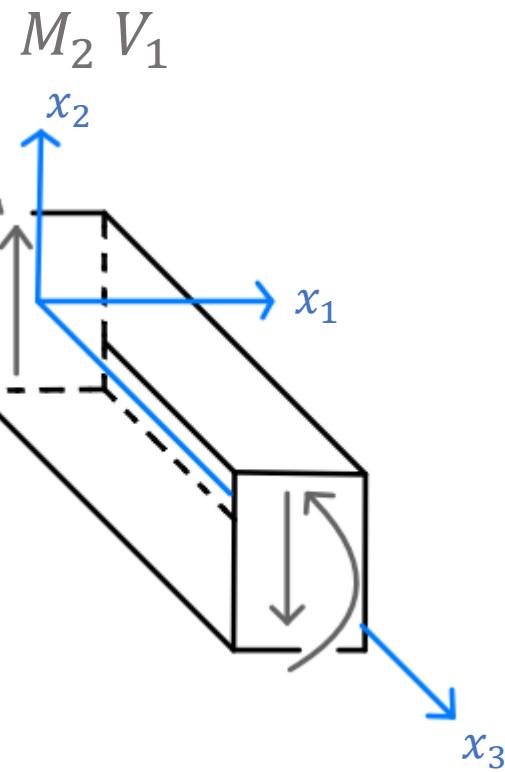
Recent research

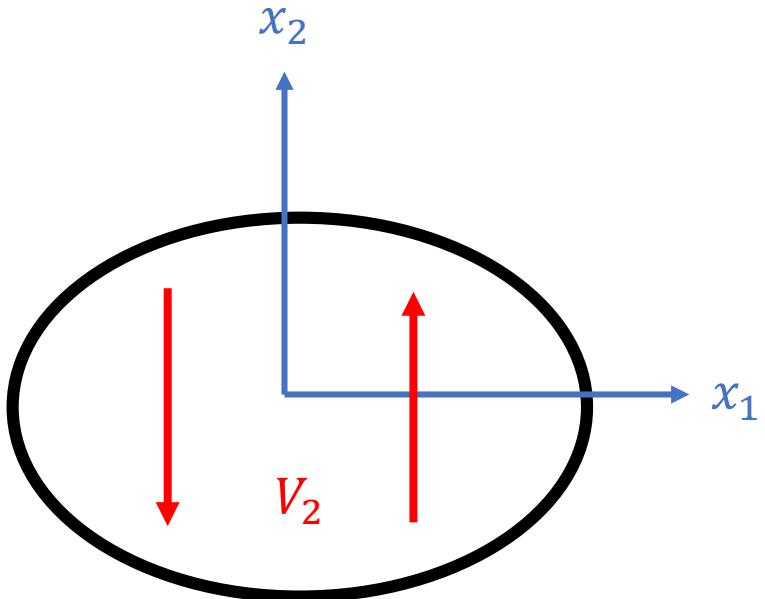
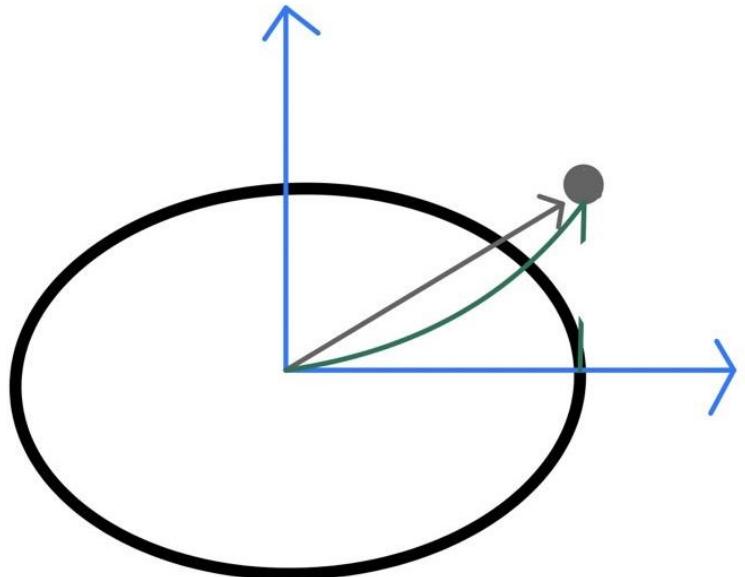


Notation



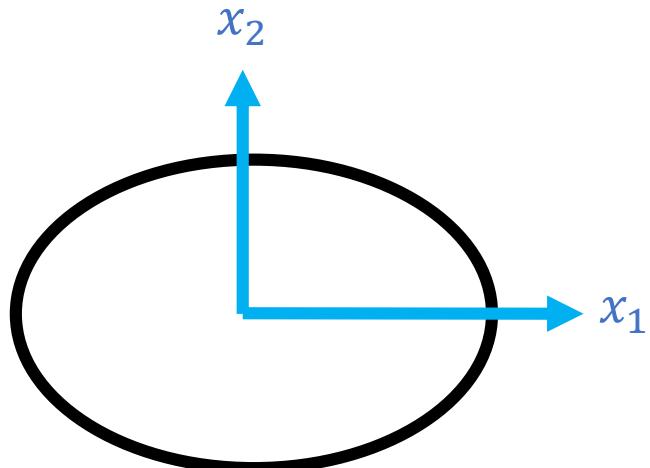
Notation





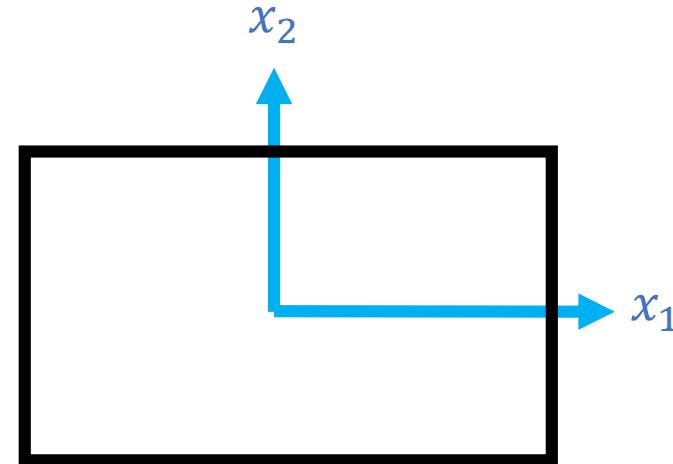
Methodology

Elliptical section



$$S = c \left(\frac{x_1^2}{a^2} + \frac{x_2^2}{b^2} - 1 \right)$$

Rectangular section



$$S = ? ??$$

Notation

x_1, x_2, x_3 : location

F_1, F_2, T_D : Free end concentrated load
& torsion

T_s : Saint-Venant torsion

T_w : warping torsion

K : section factor

w : displacement in z-direction

ϕ : angle of twist

I : second axial moment of area

$I_{ww} = \int w^2 dF$: sectorial moment of inertia

f : warping factor

E : Young's modulus

G : Shear modulus of elasticity



Official Taiwan University



Warping function: ellipse

$$\sigma_{13} = \frac{\partial S}{\partial x_2} = -\frac{2a^2}{a^2 + b^2} G \phi'_3 x_2$$

$$\sigma_{23} = \frac{\partial S}{\partial x_1} = \frac{2b^2}{a^2 + b^2} G \phi'_3 x_1$$

$$\sigma_{13} = G \phi'_3 \left(\frac{\partial W_3}{\partial x_1} - x_2 \right)$$

$$\sigma_{23} = G \phi'_3 \left(\frac{\partial W_3}{\partial x_1} + x_1 \right)$$

$$W_3 = \frac{-a^2 + b^2}{a^2 + b^2} x_1 x_2$$



國立
臺灣大學

HSV

National Taiwan University



WCCM-ENACM
VANCOUVER 2024

Future work

1. Different boundary conditions
2. Different location of loads
3. Yield conditions
4. Thin-walled cross section & Shear center